
Plan

Periodic (5 Year) Review and Revision, Run-On and Runoff Control System Plan, CCR Landfill, Reid Gardner Station

PREPARED FOR: File
PREPARED BY: NV Energy, Jacobs
DATE: October 15, 2021

1.0 INTRODUCTION

This plan documents the first five year (quinquennial) periodic review and revision of the Run-on and Runoff Control System Plan (Plan) (CH2M HILL, Inc., 2016a) for the existing coal combustion residuals (CCR) landfill (landfill) at the Reid Gardner Station (Station) as required by the U.S. Environmental Protection Agency's CCR Rule. This periodic revision concludes that the existing landfill run-on and runoff controls continue to meet the CCR Rule requirements.

1.1 PURPOSE AND SCOPE

In accordance with §257.81(c)(4), Plan reviews and updates must be completed at 5-year intervals. This periodic review and revision of the initial CCR Landfill Run-On and Runoff Control System Plan included a professional engineer review of the Plan, a review of available documentation and inspection reports for changes in site conditions from those in the Plan, and an assessment to determine whether the existing run-on and runoff controls are adequate for current conditions.

1.2 SITE INFORMATION

The Station is located 50 miles northeast of Las Vegas, within the Moapa Valley and was formerly a coal-fired electric power generation facility that produced approximately 557 megawatts (MW) of power from four generating units. Units 1 through 3 were retired in 2014 and Unit 4 was retired in 2017. Station demolition was completed in 2020.

The CCR landfill is a partially lined monofill landfill that has waste placed over approximately 111 acres. The waste fill currently extends 40 or more feet above the adjacent ground surface and generally has 3:1 (horizontal to vertical) or flatter side slopes. The landfill's surface is generally graded to drain stormwater runoff to basins located at the perimeter of the landfill. The landfill consists of two different cells: a lateral expansion called the South Lateral Landfill Expansion (SLLE) and the main part of the landfill. The SLLE has a bottom liner system consisting of a single layer of high-density polyethylene (HDPE) geomembrane and some sections of the main landfill include lined areas as well.

2.0 REGULATORY OVERVIEW

The CCR Rule was published in the Federal Register on April 17, 2015, and became effective on October 19, 2015. The CCR Rule regulates the disposal of CCR as solid waste in landfills, surface impoundments, and lateral expansions under Subtitle D of the Resource Conservation and Recovery Act. The CCR Rule sets forth minimum criteria for the requirements for run-on and runoff controls of existing CCR landfills in §257.81.

As required by §257.81(a)(1), the CCR landfill run-on control system must prevent flow onto the active portion of the landfill from a 25-year, 24-hour storm. The active portion of the landfill is defined in §257.53 as “that part of the [landfill] that has received or is receiving CCR or non-CCR waste and that has not completed closure in accordance with §257.102.” The CCR Rule also requires the run-off control system be capable of collecting and controlling the volume of water produced by a 25-year, 24-hour from the active portion of the landfill (§257.81(a)(2)). Additionally, as required by §257.81(b), run-off from the active portion of the CCR landfill must be handled in accordance with the surface water requirements in §257.3-3. Generally speaking, this means that the following actions are prohibited: 1) discharge of fill or dredge material to waters of the United States, 2) discharge of pollutants that violate a National Pollutant Discharge Elimination System permit, and 3) causing non-point source pollution to waters of the United States.

Periodic Plan assessments and updates are required to be completed and placed in the operating record every 5 years from the date that the Plan was placed into the operating record (§257.81(c)(4) and §257.105(g)(3)). The Plan was certified by a professional engineer and placed in the operating record on October 17, 2016. Accordingly, this plan must be placed in the operating record by October 17, 2021, and within 30 days of placing in the operating record, the required notifications to the State Director (§257.106(d) and §257.106(g)(3)) and placement of the updated plan on the publicly accessible internet site must also be completed (§257.107(d) and §257.107(g)(3)). The initial and periodic run-on and runoff plans must be certified by a qualified professional engineer (§257.81(c)(5)).

3.0 AVAILABLE INFORMATION

The information available regarding the status and condition of the existing CCR landfill was reviewed. Information from the operating record and other sources reviewed as part of this five year assessment included the following items:

- The Plan and associated calculations and worksheets.
- Annual inspection reports for 2016 through 2020 (CH2M HILL, Inc. 2016b, 2017, 2018, 2019, 2020).
- Topographic data from 2016 (Attachment 1) and updated through 2020 (Attachment 2) described in the following sections.

4.0 INITIAL RUN-ON AND RUNOFF CONTROL PLAN (2016)

The Plan was intended to “prevent flow onto the active portion” of the landfill, and to “collect and control” the stormwater runoff produced by the “active portion” of the landfill. As required by CCR Rule, the design storm event for the initial run-on and runoff control plan was the 25-year, 24-hour storm.

Run-On: The landfill is generally higher than the adjacent ground surface around its entire perimeter. Therefore, run-on potential does not exist for the landfill, except for the landfill’s

southern boundary where the SLLE perimeter road is lower because it has been cut into the terrain approximately 5 feet. A roadside stormwater ditch has been provided to capture the cut slope's minor run-on flows and convey it around the SLLE with no adverse impact.

Runoff: A series of diversion berms, ditches, culverts/pipes, "Arizona Crossings" (sagged roadways), and stormwater basins (SWBs) are constructed within and around the landfill to control runoff. The stormwater basins include the lined contact water pond system (CWPS), which was built with the SLLE to control runoff from the landfill perimeter, and leachate from lined portions of the landfill. These features were used to collect, route and eliminate discharge from the landfill downstream in accordance with §257.81(b) and §257.3-3.

5.0 CHANGES IN CONDITIONS FROM THE PLAN

Based on review of updated topography obtained in 2020 following the placement of Station demolition construction debris in the landfill, the most significant change in the landfill formation occurred in the center of the mass. Fill has been placed in a manner that results in additional runoff to the northern boundary of the landfill. No fill was placed in the southeastern portion of the landfill, and that has created a collection area for runoff from the slope and center positioned fill and a route to the control storage basin. Additionally, the direction subbasin SUB8 drains is currently southwesterly, as opposed to the Plan, which had shown it draining southeasterly. Attachment 3 provides a map showing the current subbasins, drainage patterns, and drainage controls, (including the receiving storage basins, referred to as combination points in the attached drawings and calculations if they receive flow from multiple subbasins) overlain on the updated topography. Although topography has changed in the landfill interior, the runoff control measures identified in 2016 to collect and route stormwater at its perimeter prior to discharge continue to function in compliance with the CCR Rule. Additionally, current site conditions have not changed the run-on impacts from the Plan. Therefore, no new recommendations are made to control run-on at the site.

6.0 CHANGES IN FLOW CALCULATIONS FROM THE PLAN

Due to the formation changes, a revised hydrologic analysis was performed for the runoff potential of the landfill to estimate peak flow rates and runoff volumes required to be controlled. The revised hydrologic analysis used the U.S. Army Corps of Engineers Hydrologic Engineering Center (HEC-1) software and Soil Conservation Service (SCS) Unit Hydrograph method runoff computations. Inputs for HEC-1 and SCS computation used the Natural Resources Conservation Service Type II rainfall distribution for a 24-hour storm event, and the same hydrologic parameters used in the Clark County standard form 4 (SF4) calculations in the Plan were also used for this revised analysis. One adjustment was made to the hydrologic parameters, revising the curve number ("CN" on the SF4 column headers) from 86 to 83. The SF4 calculation summary and the HEC-1 calculations are presented in Attachment 4. The results of 2021 and 2016 calculations are summarized in Table 6.1.

Periodic (5-year) Review and Revision, Run-On and Runoff Control System Plan,
CCR Landfill, Reid Gardner Station

Table 6.1. Summary of Subbasin Changes – 2021 vs. 2016		
Subbasin ID	2021 Current Topography Q₂₅ (cfs)	2016 Plan Q₂₅ (cfs)
SUB1	16	22
SUB2	6	7
SUB2A	9	12
SUB2B	5	6
SUB2C	4	5
SUB3	4	6
SUB3A	2	3
SUB4	15	9
SUB5	3	4
SUB5A	2	2
SUB6	10	13
SUB7	5	7
SUB8	3	5
SUB9	4	5
SUB10	3	4
SUB11	7	8
SUB12	2	2
SUB12A	3	4
SLLE	14	19

Notes:

cfs = cubic feet per second

ID = identifier

Q₂₅ = Discharge from design flood (24- hour, 25-year)

SUB = subbasin drainage basin

The total runoff to the combination points on the site have also been compared to the 2016 Plan and summarized in Table 6.2.

Table 6.2. Summary of Combination Point Changes – 2021 vs. 2016		
CP ID	2021 Current Topography Q₂₅ (cfs)	2016 Plan Q₂₅ (cfs)
CWPS1	20	32
CWPS	32	49
SWB1	21	27
SWB4	17	12
SWB5	15	19
SWB8	10	8

Notes:

CP = Combination Points

ID = identifier

Q₂₅ = Discharge from design flood (24- hour, 25-year)

CWPS = contact water pond system

SWB = stormwater basin

7.0 EVALUATION OF RUN-ON AND RUNOFF CONTROLS

As stated in Section 5.0 of this plan, the run-on controls remain in their 2016 state and there are no new impacts by offsite flow anticipated based on current topography. However, due to formation changes within the landfill, an evaluation was done on the existing runoff control facilities within the limits of the changed subbasins.

Ditches, Berms, and “Arizona Crossing”: Utilizing the current topography, normal depth hydraulic calculations were used to determine the capacities of the subject routing facilities. Based on capacity calculations and the flow rates realized by the tributary subbasin(s), no changes to ditches and berms are necessary and they function in accordance with CCR Rule landfill requirements to convey the storm flows to the applicable stormwater basins. The hydraulic calculations showing maximum capacity of each feature have been included in Attachment 5, with the applicable name of each facility shown on the subbasin map, included in Attachment 3. Attachment 5 presents individual worksheets for each runoff conveyance or control feature described below. Each worksheet indicates the capacity of the feature and in all cases the capacity was greater than the calculated runoff discharge. For example, the first calculation sheet, for the CWPS shows a capacity of 75 cfs compared to the design flood discharge in Table 6.2 above of 32 cfs. Note for the control features that are intended to act as positive drainage to the correct stormwater basin, only an inspection of current topography was performed to determine the applicable feature remained in a similar state as proposed in the Plan.

Culverts: The current topography around the culverts shows similar ground elevations to those presented in the Plan. The flow rate to the 24-inch reinforced concrete pipes (RCP) culvert into Stormwater Basin 5 has not increased from the 2016 Plan; therefore, no change is needed to this facility. The current peak flow rate to the 18-inch RCP culvert into Stormwater Basin 8 has slightly increased to 10 cfs from 8 cfs; however, the culvert remains properly sized to accept/convey flow per the design intent and no change is required to this facility. The updated HY-8 culvert capacity calculation for the 18-inch RCP culvert into Stormwater Basin 8 has been included in Attachment 5.

Stormwater Collection Basins: The current topography has been studied in the locations of the stormwater basins to ensure adequate capacity of storage volume is provided to meet requirements of the CCR Rule. All the stormwater basins remain in their previous configuration to provide the required volume, except for SWB4. To provide the required capacity of SWB4, the assumed footprint of the storage area was increased from the 2016 Plan; the existing berm that straddles the northern boundary at this location provides the required volume. Refer to the footprint exhibit of SWB4 volume in the last page of Attachment 5 for an illustration of the updated catchment area provided by the current topography. Note, no physical modification to the basin was required, the change was only to recognize the larger basin volume as it already exists. Table 7.1 represents the required volume routed to each of the stormwater basins, compared to the actual volume provided in the current topography. The basins do not require any expansion or adjustment based on current topography.

Table 7.1. Summary of Required Volumes in Stormwater Basins				
Stormwater Basin ID <i>(Relevant HEC-1 ID)</i>	2021 HEC-1 Volume (ac-ft)	2021 HEC-1 Volume (yd³)	Provided Topography Volume (yd³)	Additional Volume Required (yd³)
CWPS <i>(CWPS)</i>	2.59	4,178	5,440	0
SWB1 <i>(SWB1)</i>	1.69	2,727	2,818	0
SWB3 <i>(SUB3A)</i>	0.16	258	366	0
SWB4 <i>(SWB4)</i>	1.52	2,452	4,858	0
SWB5 <i>(SWB5)</i>	1.15	1,855	2,743	0
SWB6 <i>(SUB12)</i>	0.12	194	223	0
SWB7 <i>(SUB12A)</i>	0.20	323	428	0
SWB8 <i>(SWB8)</i>	0.60	968	980	0

Note:

ac-ft = acre-feet

yd³ = cubic yard

8.0 CONCLUSION

Based on review of the current topography, run-on continues to have no impact to the landfill. A reanalysis of the subbasins within the active landfill area shows that flow rates have changed since the Plan was submitted, but the hydraulic calculations of the runoff control facilities show there is adequate capacity for the re-division of storm flows within the area in the ditches, berms, and culverts. Additionally, the stormwater basins that receive additional runoff (when compared to the Plan), have been determined to provide adequate storage capacity in their vicinity to meet criteria required for a CCR landfill.

It is recommended that NV Energy continue its annual inspections, and maintenance of the landfill area, including the existing stormwater control features, to maintain compliance with runoff control requirements.

9.0 CERTIFICATION

This quinquennial assessment of the run-on and runoff control plan was conducted in accordance with the requirements of §257.8 of the CCR Rule.



----seal----

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DN: C=US, E=steve.jones1@jacobs.com, O=Jacobs, CN="Stephen M. Jones, P.E."
Date: 2021.10.15 10:02:46-07'00'

The originally certified document inadvertently included Adobe comment call-outs on three map attachments. After the certification above, the call-outs were removed. No other changes were made, as certified below.



Stephen M. Jones, P.E. Digitally signed by Stephen M. Jones, P.E.
DN: C=US, E=steve.jones1@jacobs.com, O=Jacobs, CN="Stephen M. Jones, P.E."
Date: 2021.10.28 10:24:15-07'00'

10.0 REFERENCES

CH2M HILL, Inc. 2016a. *Run-on and Runoff Control System Plan, Coal Combustion Residuals Landfill, Reid Gardner Generating Station*. October 17.

CH2M HILL, Inc. 2016b. *Annual Coal Combustion Residuals Landfill Inspection, Reid Gardner Generating Station*. December 14.

CH2M HILL, Inc. 2017. *Annual Coal Combustion Residuals Landfill Inspection, Reid Gardner Generating Station*. December 11.

CH2M HILL, Inc. 2018. *Annual Coal Combustion Residuals Landfill Inspection, Reid Gardner Generating Station*. November 20.

CH2M HILL, Inc. 2019. *Annual Coal Combustion Residuals Landfill Inspection, Reid Gardner Generating Station*. November 21.

CH2M HILL, Inc. 2020. *Annual Coal Combustion Residuals Landfill Inspection, Reid Gardner Generating Station*. December 1.

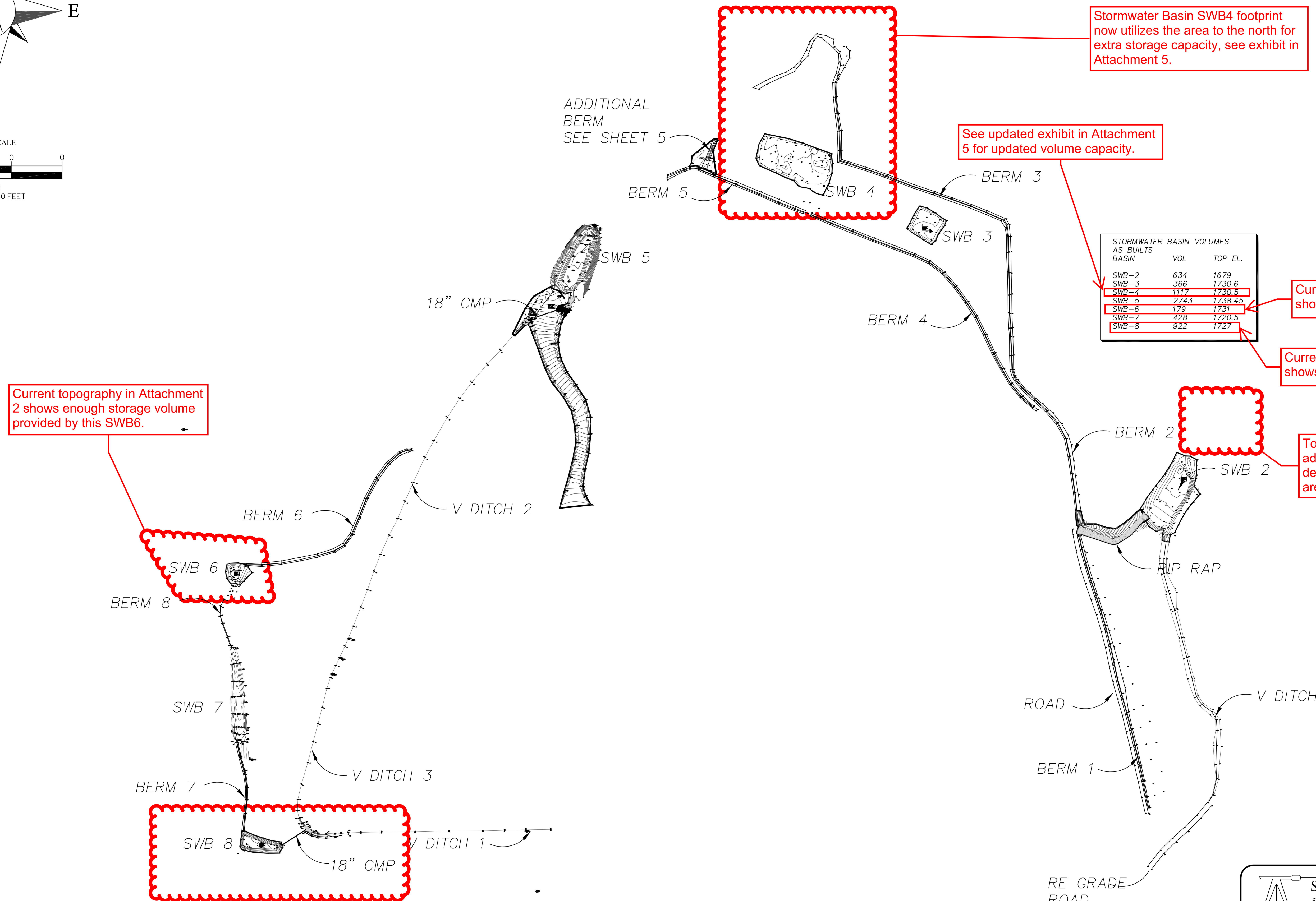
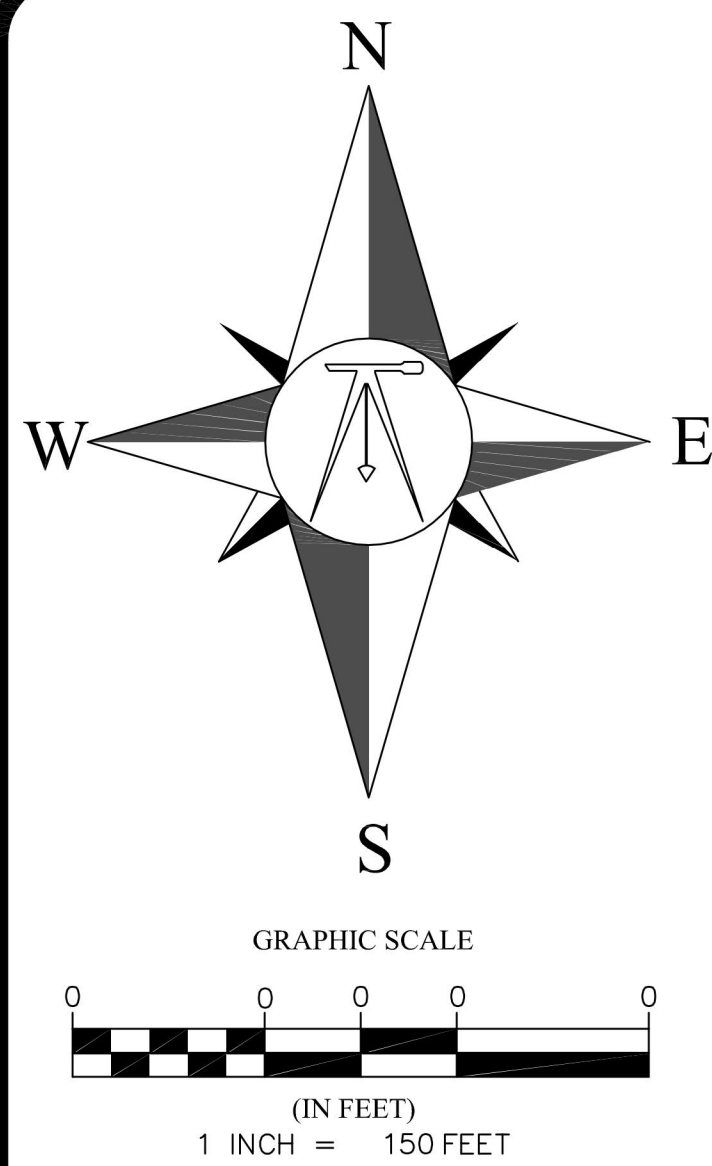
11.0 ATTACHMENT LIST

1. Referenced/Annotated 2016 As-Built Stormwater Features Survey and Design Drawings
2. Current Site Topography
3. 2021 Updated Landfill Runoff Subbasin Map
4. 2021 Updated Landfill Runoff Hydrology Calculations
5. Updated Runoff AND Run-On Hydraulic Calculations

Attachment 1

Referenced/Annotated 2016 As-Built Stormwater Features Survey and Design Plans

As-Built Features



STORMWATER BASIN VOLUMES AS BUILTS		
BASIN	VOL	TOP EL.
SWB-2	634	1679
SWB-3	366	1730.6
SWB-4	1117	1730.5
SWB-5	2743	1738.45
SWB-6	179	1731
SWB-7	428	1720.5
SWB-8	922	1727

Stormwater Basin SWB4 footprint now utilizes the area to the north for extra storage capacity, see exhibit in Attachment 5.

See updated exhibit in Attachment 5 for updated volume capacity.

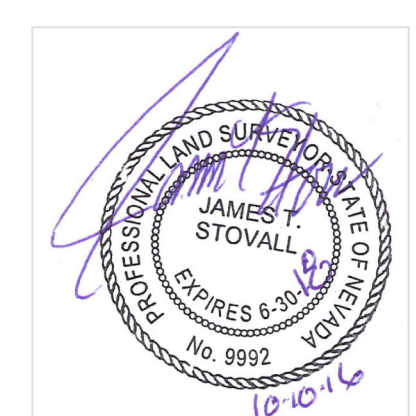
Current topography in Attachment 2 shows adequate storage capacity.

Current topography in Attachment 2 shows adequate storage capacity.

Total volume for SWB1 includes additional storage area (see design drawings) plus SWB2 area shown here.

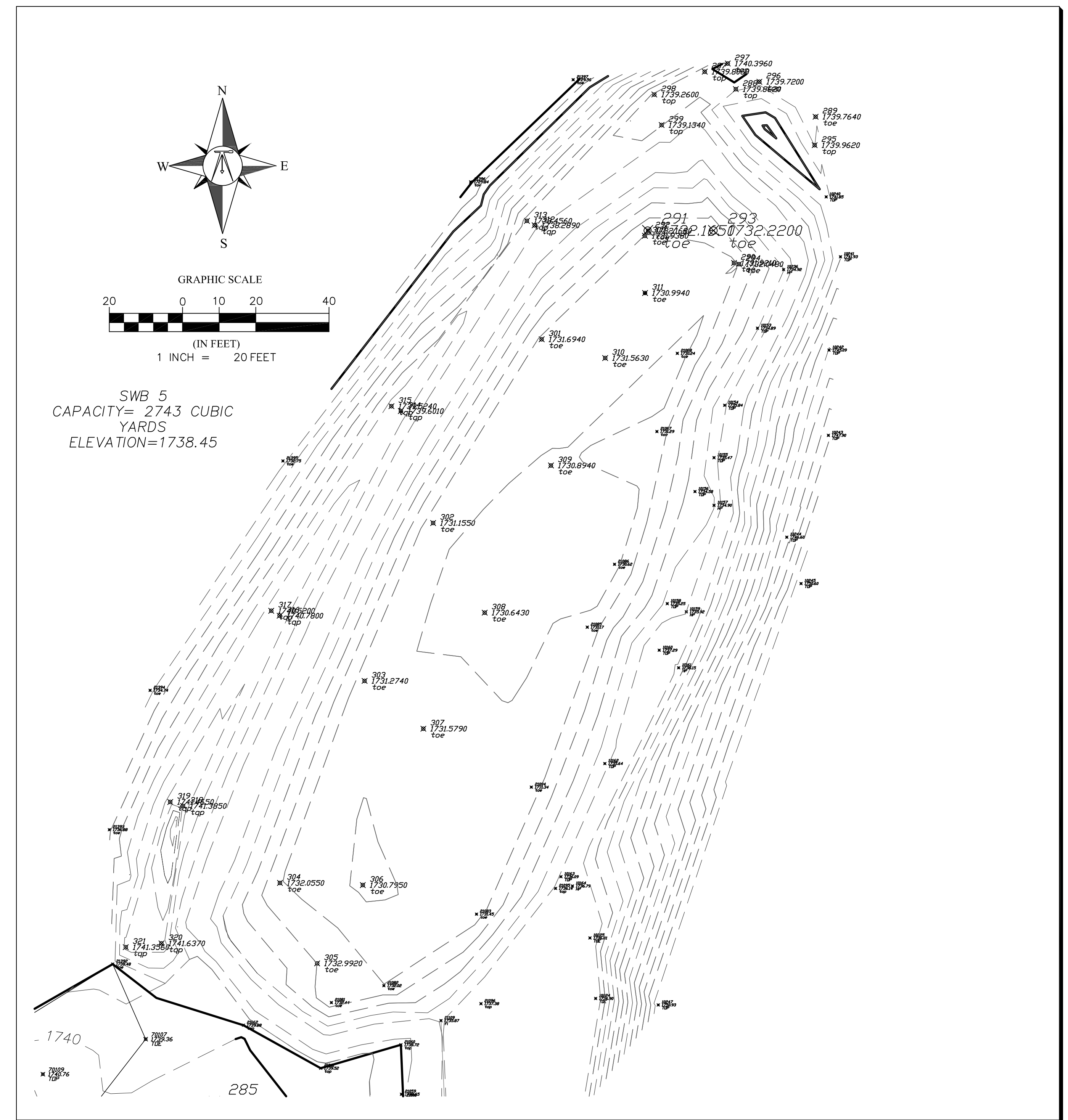
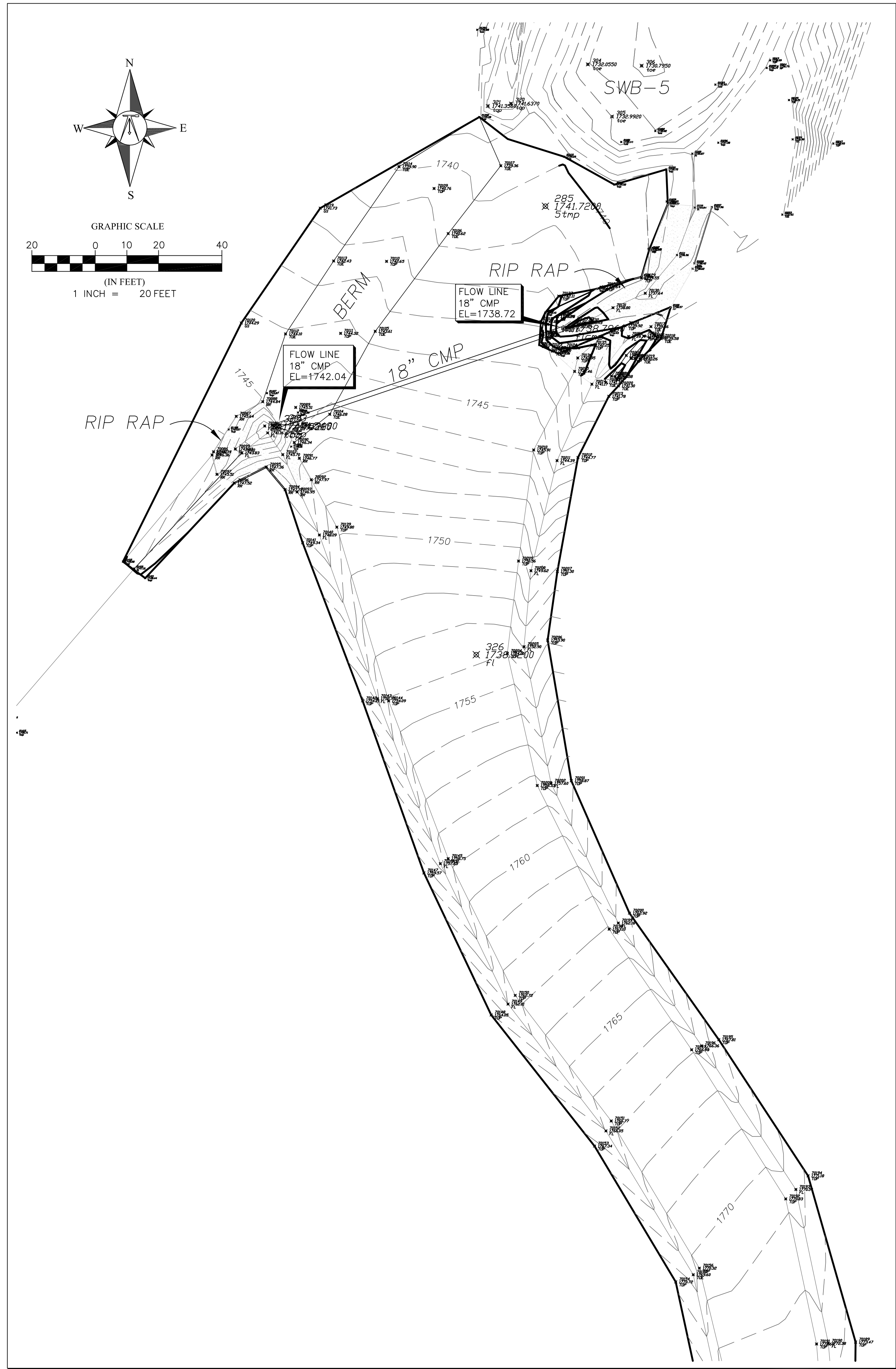
Current topography in Attachment 2 shows enough storage volume provided by this SWB6.

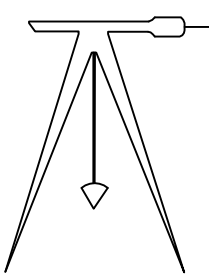
Current topography in Attachment 2 shows enough storage volume provided by this SWB8.



 STOVALL LAND SURVEYING 5840 W. CRAIG ROAD, STE. 120-346 LAS VEGAS, NEVADA 89130 (702)419-3432 stovallsurvey@gmail.com			
REID GARDNER CCR LANDFILL RUN-ON AND RUN-OFF CONTROL SYSTEM AS-BUILTS			
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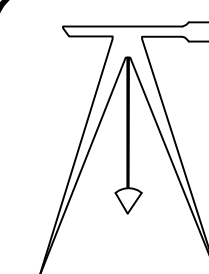
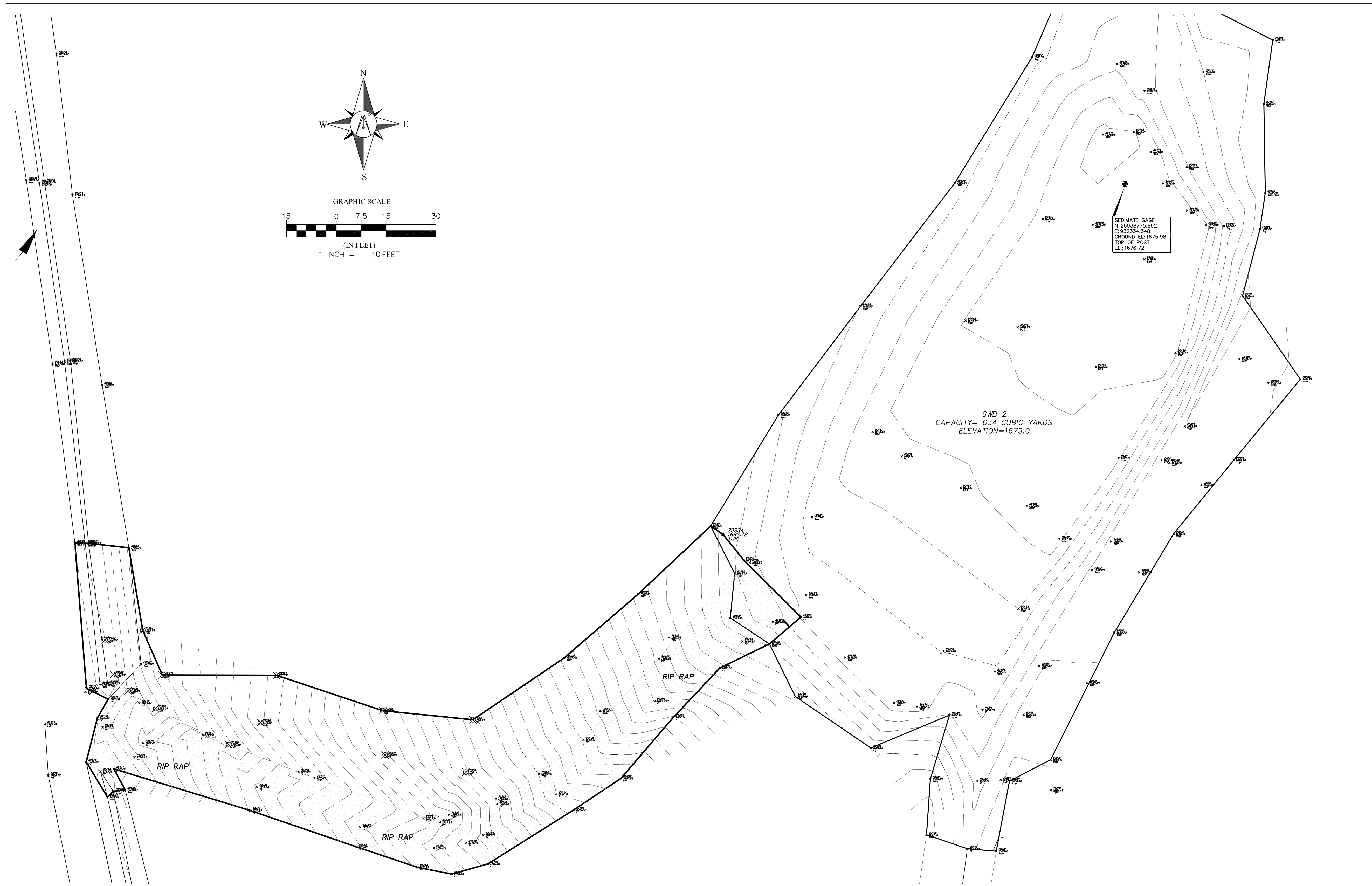




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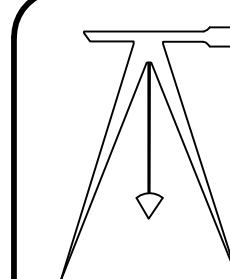
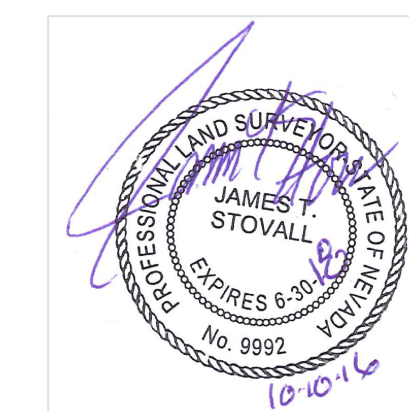
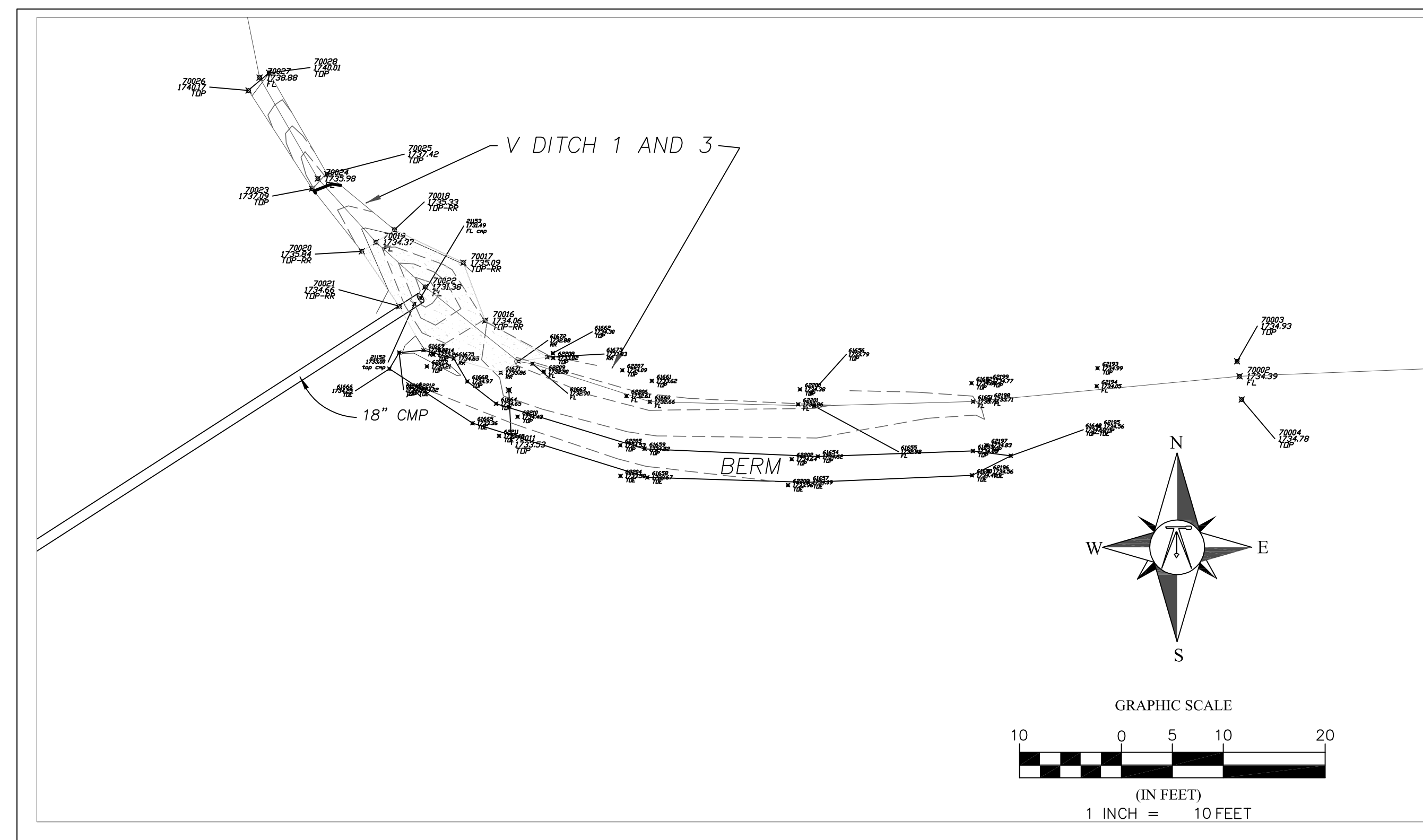
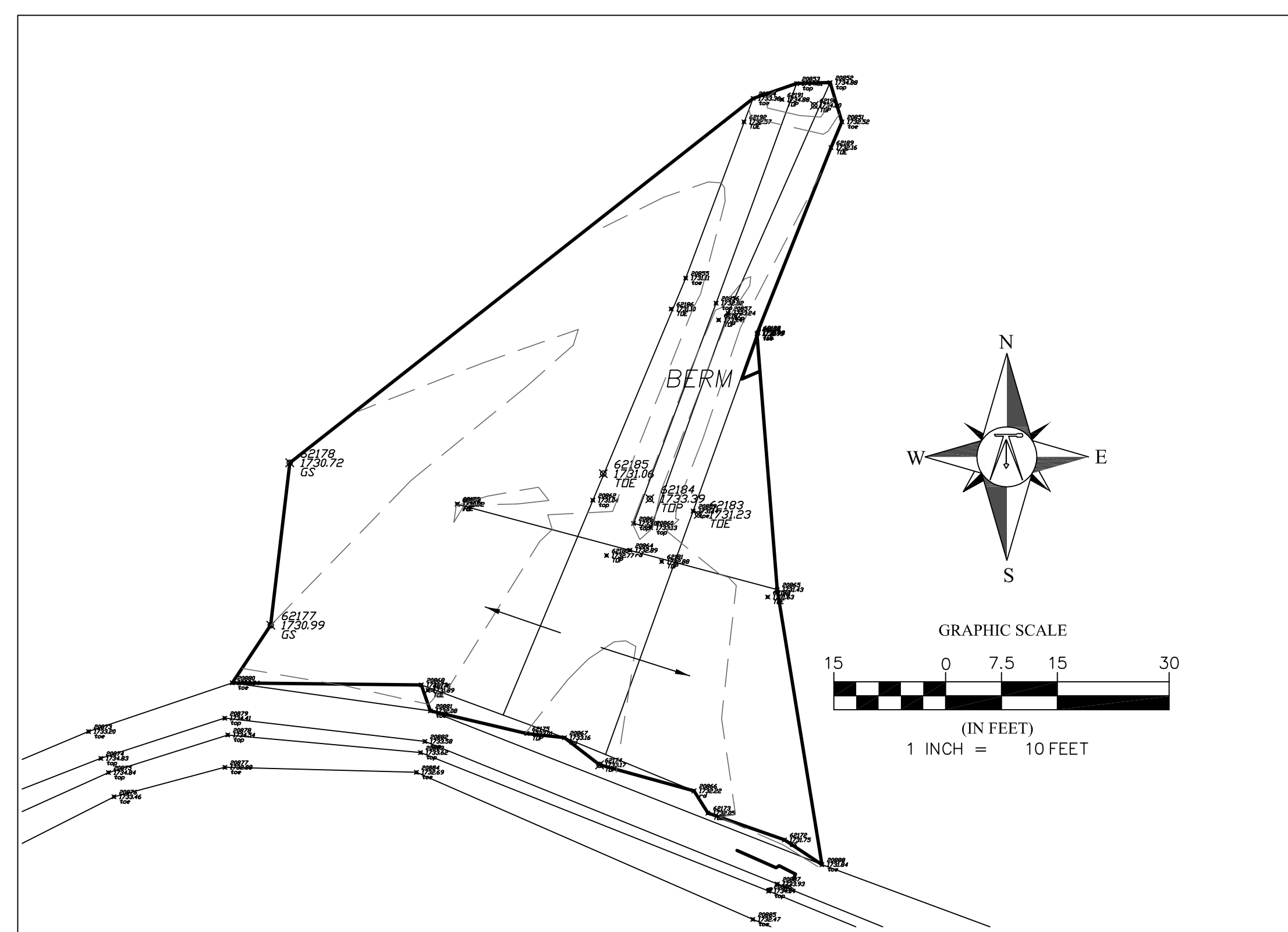
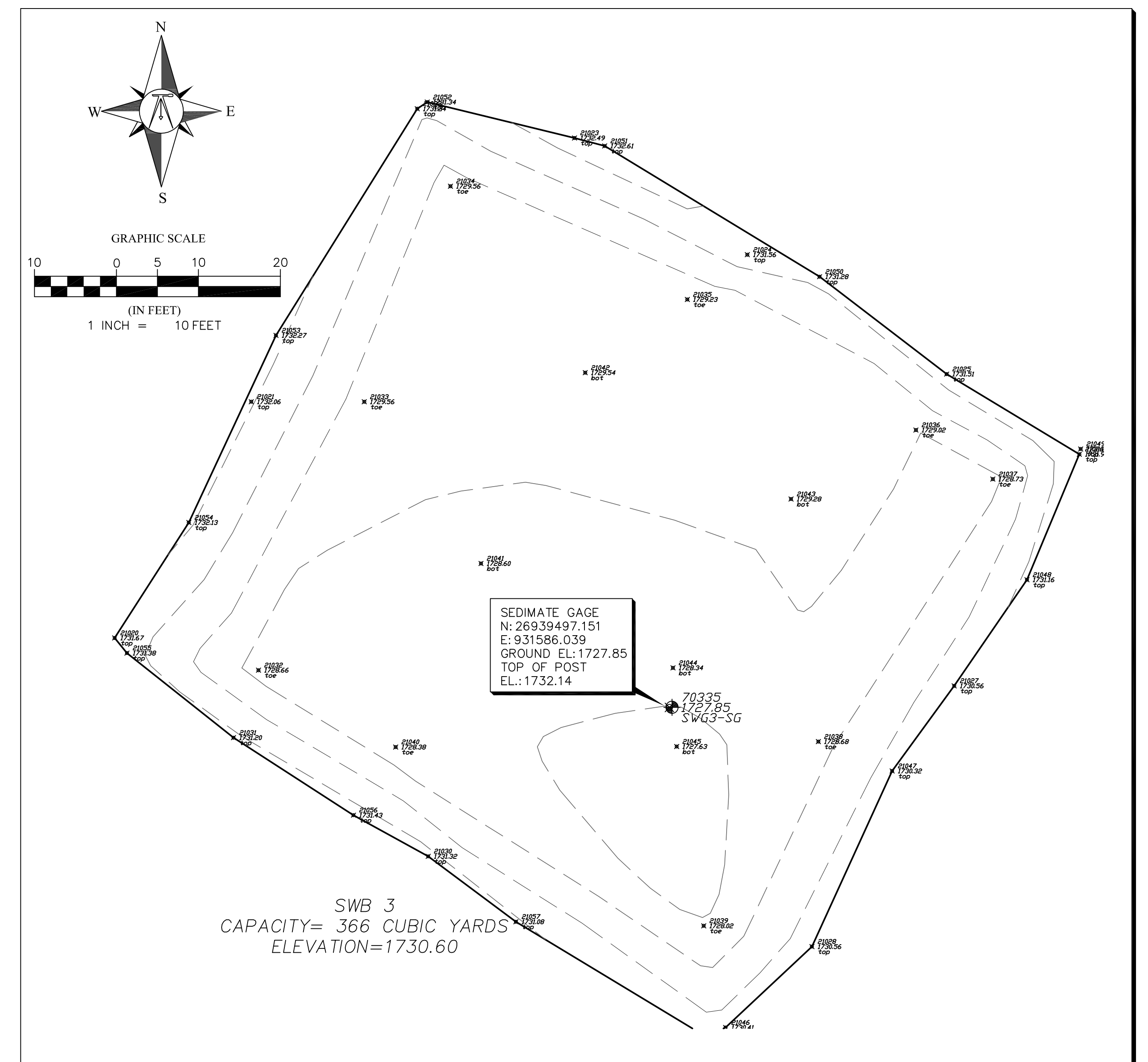
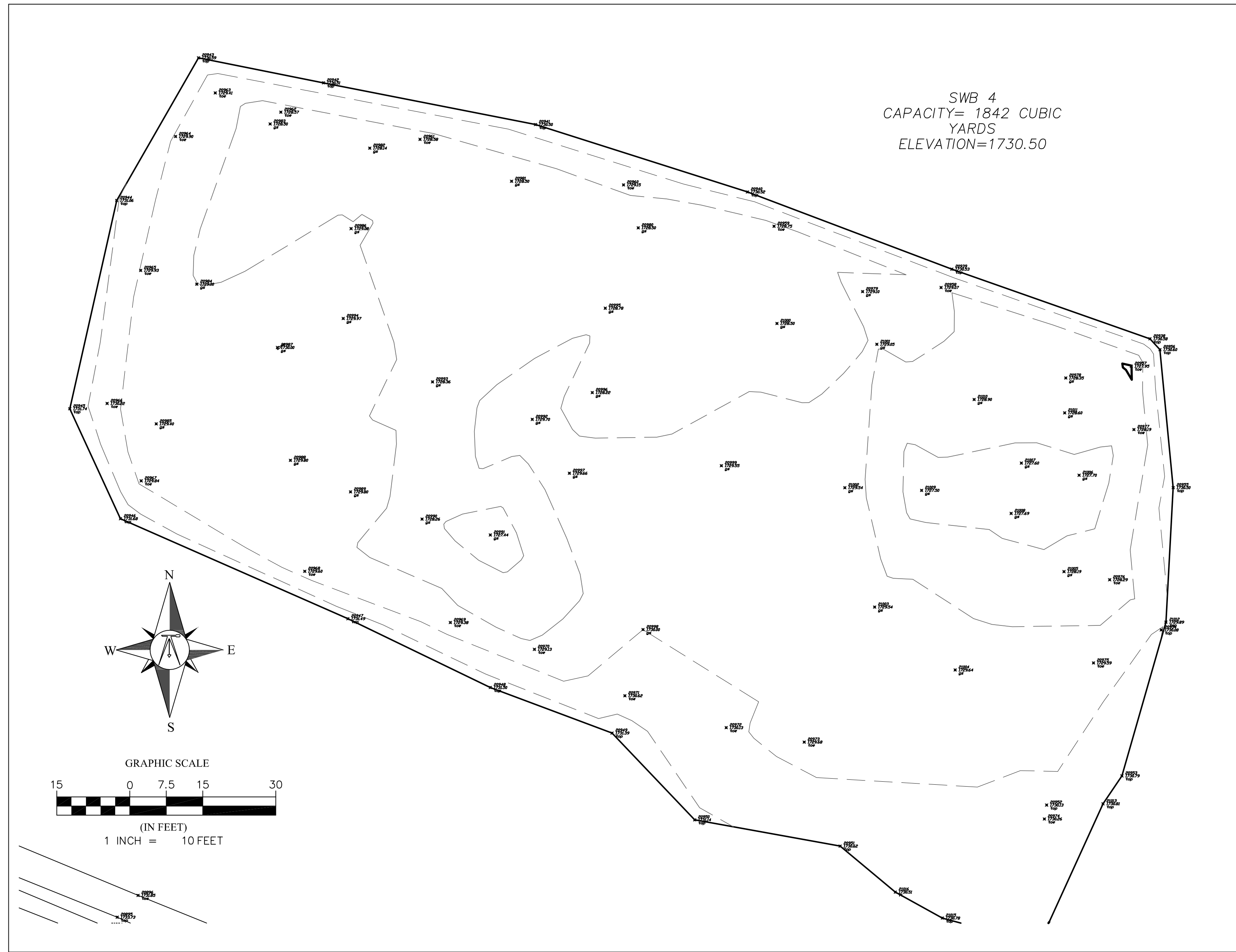


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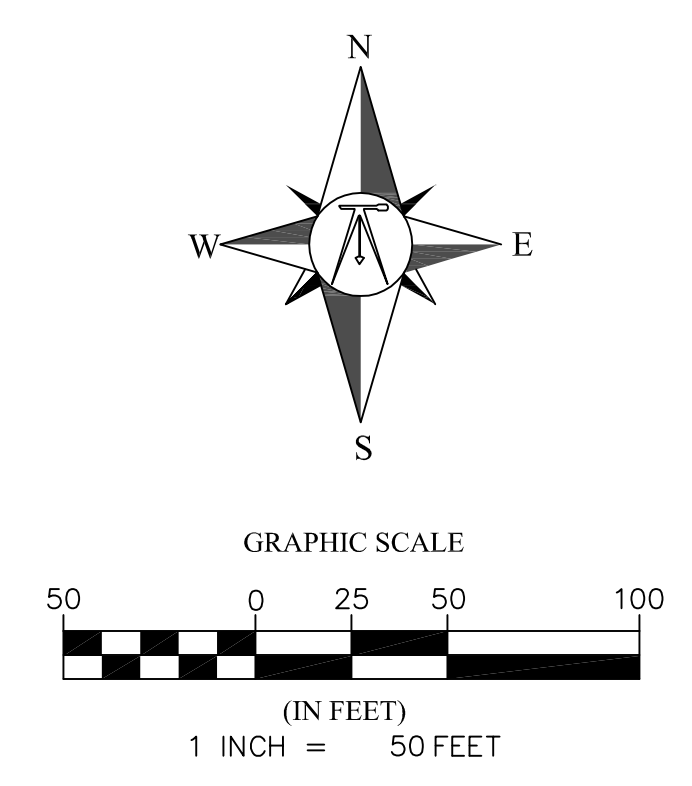
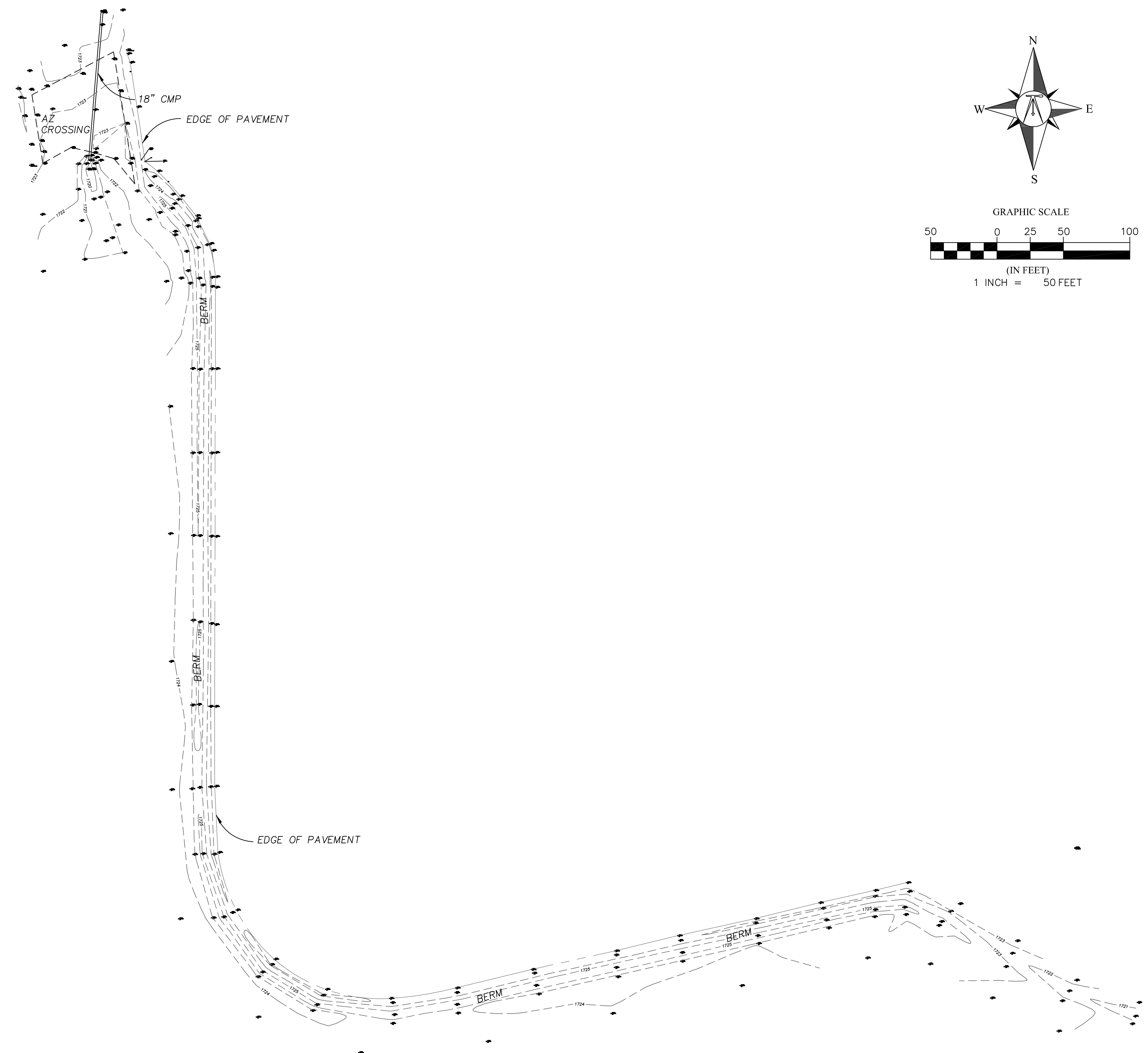


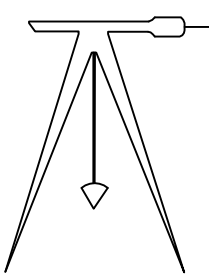
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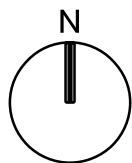


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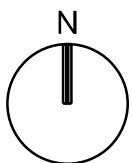
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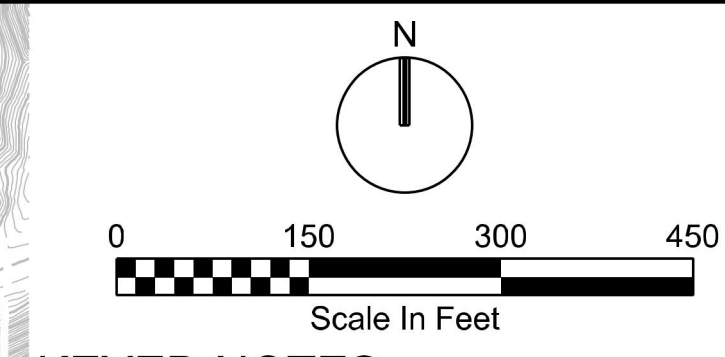
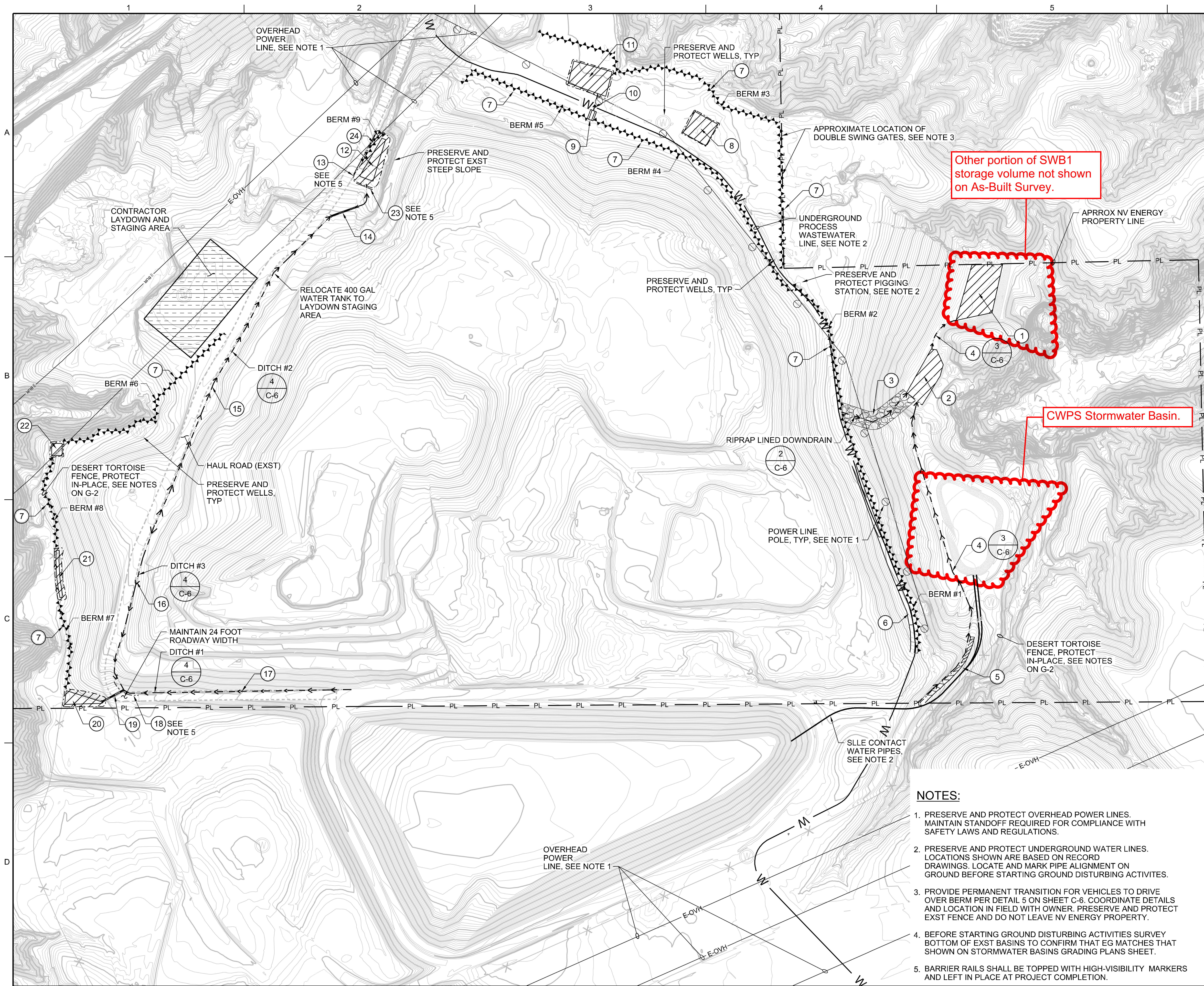
Design Drawings



SHEET 1 OF 9	G-1	COVER AND INDEX SHEET
SHEET 2 OF 9	G-2	GENERAL NOTES AND LEGEND SHEET
SHEET 3 OF 9	C-1	OVERALL SITE PLAN AND CIVIL PLAN
SHEET 4 OF 9	C-2	SURVEY CONTROL PLAN
SHEET 5 OF 9	C-3	STORM WATER BASIN GRADING PLAN
SHEET 6 OF 9	C-4	DRAINAGE PROFILES
SHEET 7 OF 9	C-5	ROADWAY PLAN & PROFILE
SHEET 8 OF 9	C-6	DETAILS
SHEET 9 OF 9	C-7	SEDIMENT DEPTH GAGE ELEVATION & DETAIL



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KEYED NOTES:

- 1 REGRADE EXISTING STORMWATER BASIN. SEE SHEET C-3, DETAIL SWB-1 FOR SURVEY CONTROL INFORMATION.
- 2 REGRADE EXISTING STORMWATER BASIN. SEE SHEET C-3, DETAIL SWB-2 FOR SURVEY CONTROL INFORMATION.
- 3 CONSTRUCT RIPRAP LINED DOWNDRAIN. SEE SHEET C-2 FOR SURVEY CONTROL INFORMATION. SEE DETAIL 2 ON SHEET C-6.
- 4 CLEAN-OUT DEBRIS AND SEDIMENT FROM EXISTING DITCH
- 5 REGRADE ROAD. SEE SHEET C-5 FOR SURVEY CONTROL INFORMATION.
- 6 REGRADE ROAD. SEE SHEET C-5 FOR SURVEY CONTROL INFORMATION.
- 7 CONSTRUCT BERM. SEE SHEET C-2 FOR SURVEY CONTROL INFORMATION. SEE DETAIL 1 ON SHEET C-6.
- 8 CONSTRUCT STORMWATER BASIN. SEE SHEET C-3, DETAIL SWB-3 FOR SURVEY CONTROL INFORMATION.
- 9 CONSTRUCT ARIZONA CROSSING. SEE SHEET C-7, DETAIL 3.
- 10 CONSTRUCT V-DITCH. SEE DETAIL 3, SHEET C-6. SEE SHEET C-2 FOR CONTROL LINE INFORMATION
- 11 CONSTRUCT STORMWATER BASIN. SEE SHEET C-3, DETAIL SWB-4 FOR SURVEY CONTROL INFORMATION.
- 12 CONSTRUCT STORMWATER BASIN. SEE SHEET C-3, DETAIL SWB-5 FOR SURVEY CONTROL INFORMATION.
- 13 INSTALL APPROX 232 LF TYPE FA CONCRETE BARRIER RAIL PER NDOT STD DWG R-8.6.2. SEE NOTE 5.
- 14 INSTALL 24" RCP PER NDOT STD SPEC SEC 603. SEE DITCH #2 PROFILE ON SHT C-4. INSTALL CLASS 300 CULVERT INLET AND OUTLET PROTECTION PER NDOT STD DWG R-3.1.5 & NDOT STD SPEC SEC 610.
- 15 16 17 CONSTRUCT ROADSIDE V-DITCH. SEE SHEET C-4 FOR PROFILE. SEE SHEET C-2 FOR SURVEY CONTROL INFORMATION. SEE DETAIL 4 ON SHEET C-6.
- 18 INSTALL APPROX 97 LF TYPE FA CONCRETE BARRIER RAIL PER NDOT STD DWG R-8.6.2. SEE NOTE 5.
- 19 INSTALL 18" RCP PER NDOT STD SPEC SEC 603. SEE DITCH #1 PROFILE ON SHT C-4. INSTALL CLASS 300 CULVERT INLET AND OUTLET PROTECTION PER NDOT STD DWG R-3.1.5 & NDOT STD SPEC SEC 610.
- 20 CONSTRUCT STORMWATER BASIN. SEE SHEET C-3, DETAIL SWB-8 FOR SURVEY CONTROL INFORMATION.
- 21 CONSTRUCT STORMWATER BASIN. SEE SHT C-3, DETAIL SWB-7 FOR SURVEY CONTROL INFORMATION.
- 22 CONSTRUCT STORMWATER BASIN. SEE SHEET C-3, DETAIL SWB-6 FOR SURVEY CONTROL INFORMATION.
- 23 INSTALL APPROX 21 LF TYPE FA CONCRETE BARRIER RAIL PER NDOT STD DWG R-8.6.2. SEE NOTE 5.
- 24 CONSTRUCT BERM. SEE SHEET C-2 FOR SURVEY CONTROL INFORMATION. SEE DETAIL 1 ON SHEET C-6.

NOTES:

1. PRESERVE AND PROTECT OVERHEAD POWER LINES. MAINTAIN STANDOFF REQUIRED FOR COMPLIANCE WITH SAFETY LAWS AND REGULATIONS.
2. PRESERVE AND PROTECT UNDERGROUND WATER LINES. LOCATIONS SHOWN ARE BASED ON RECORD DRAWINGS. LOCATE AND MARK PIPE ALIGNMENT ON GROUND BEFORE STARTING GROUND DISTURBING ACTIVITIES.
3. PROVIDE PERMANENT TRANSITION FOR VEHICLES TO DRIVE OVER BERM PER DETAIL 5 ON SHEET C-6. COORDINATE DETAILS AND LOCATION IN FIELD WITH OWNER. PRESERVE AND PROTECT EXST FENCE AND DO NOT LEAVE NV ENERGY PROPERTY.
4. BEFORE STARTING GROUND DISTURBING ACTIVITIES SURVEY BOTTOM OF EXST BASINS TO CONFIRM THAT EG MATCHES THAT SHOWN ON STORMWATER BASINS GRADING PLANS SHEET.
5. BARRIER RAILS SHALL BE TOPPED WITH HIGH-VISIBILITY MARKERS AND LEFT IN PLACE AT PROJECT COMPLETION.

ISSUED FOR CONSTRUCTION	NO.	03/16/16	DATE	03/16/16
REVISION	CHK	J. GRIEST	DR	S. BOURQUE
BY	APVD	N. BETTS	APVD	N. BETTS
MS	BY	APVD	MS	BY
NB	BY	APVD	NB	BY

2485 VILLAGE VIEW DRIVE, SUITE 350
HENDERSON, NEVADA 89074
PHONE: 702-369-6175

REID GARDNER STATION
Moapa, Nevada

CCR LANDFILL RUN-ON AND RUN-OFF CONTROL SYSTEM, WO # 9837520901

ch2m

OVERALL SITE PLAN AND CIVIL PLAN

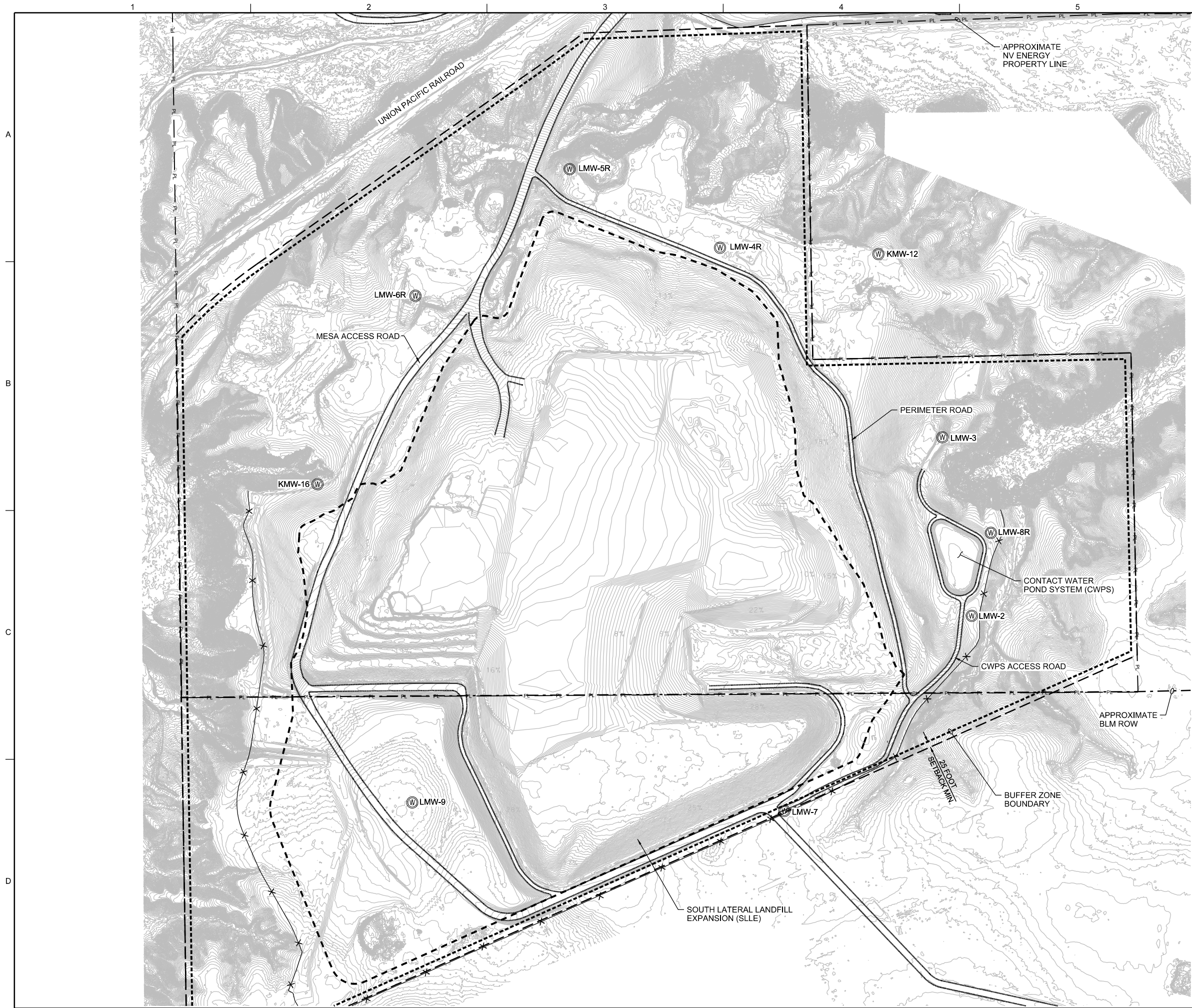
VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING.
0" 1"

DATE	MARCH 2016
PROJ	670439
DWG	C-1
SHEET	3 OF 9

ISSUED FOR CONSTRUCTION

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Attachment 2
Current Site Topography



LEGEND

- PL — PL — APPROXIMATE PROPERTY LINE
- - - - - APPROXIMATE BLM ROW
- - - - - LINED AREA
- - - - - DISPOSAL SITE BOUNDARY
- - - - - BUFFER ZONE BOUNDARY
- - - - - APPROXIMATE LIMIT OF ACTIVE FILL AREA PER 2011 LANDFILL PERMIT
- ⊙ GROUND WATER MONITORING WELL
- X - X - TORTOISE FENCE

JACOBS

REMEDIAL EXCAVATION PROJECTS

LANDFILL FILL PLAN PHASE 2

MATERIAL REMOVAL FROM ASH FILL REMOVAL PROJECTS

1301 N. GREEN VALLEY PKWY, SUITE 200
HENDERSON, NEVADA 89074
PHONE: 702-389-6175

REID GARDNER STATION
Moapa, Nevada

VERIFY SCALE

BAR IS ONE INCH ON ORIGINAL DRAWING.

DATE JULY 2021

PROJ 705712

DWG C-15

SHEET 17 OF 19

ISSUED FOR CONSTRUCTION

07/21/21

NO. 0

DATE 07/21/21

DSGN S. BOURQUE

DR R. VILORIA

CHK R. DRESEL

REVISION

RD BY

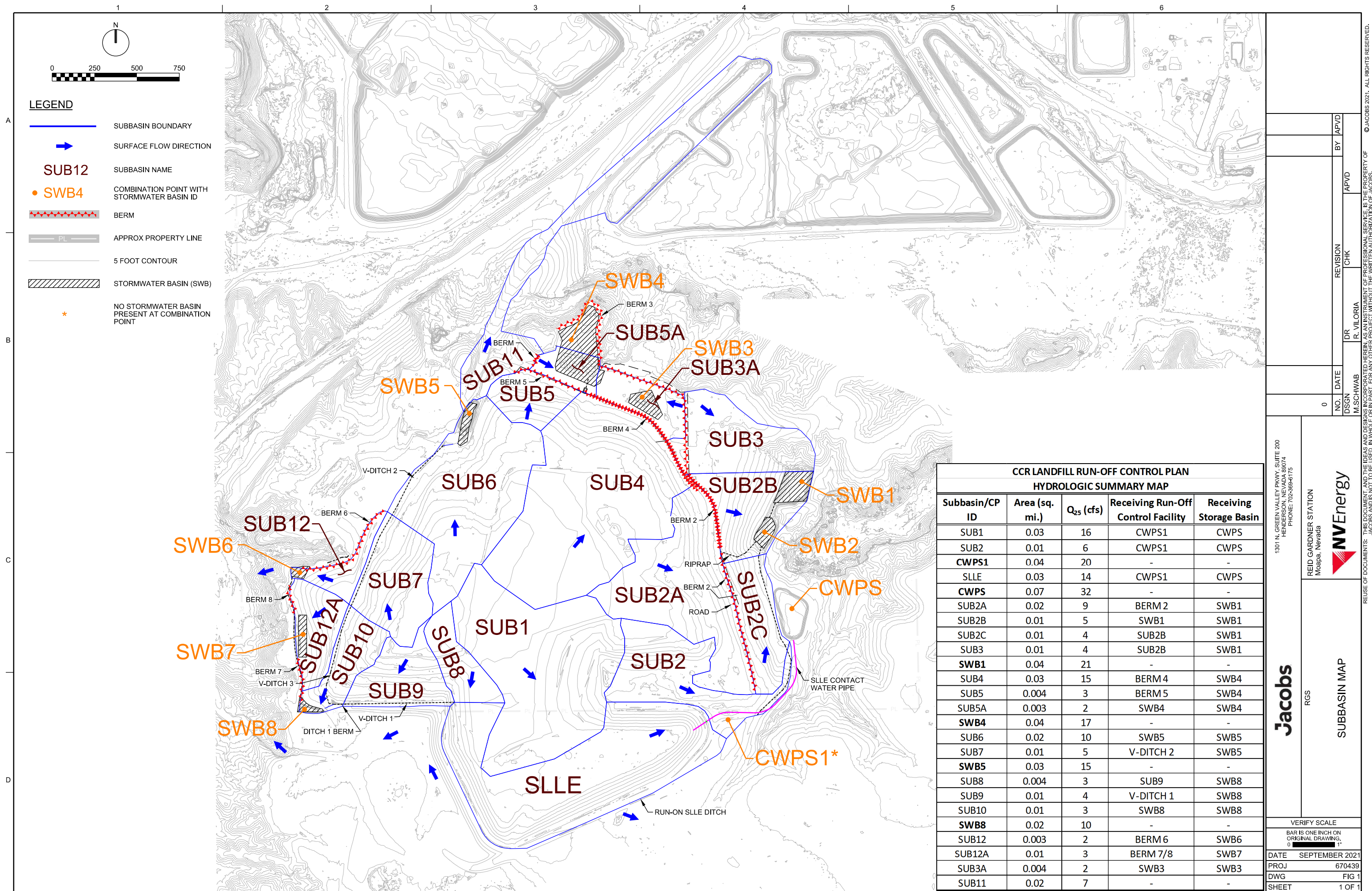
SD APVD

APVD N. BETTIS

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PW-WBG:387230_NV ENERGY\ New_Projct_September_2021.dgn PLOT DATE: 2021/10/14 PLOT TIME: 1:06:21 PM

Attachment 3
2021 Updated Landfill Runoff Subbasin Map



Attachment 4
2021 Updated Landfill Runoff Hydrology Calculations

SF4 Calculations

TIME OF CONCENTRATION / LAG TIME DETERMINATION - less than 1 mi²



CCR LANDFILL STORMWATER CONTROLS

RUN-OFF SUBBASIN HYDROLOGIC PARAMETERS

Project No: NVE01918

Date: September-21

Calculated by: FA

SUB-BASIN DATA					INITIAL / OVERLAND TIME (Ti)			TRAVEL TIME (Tt)							T _{lag}	REMARKS
Basin ID	DEV./UNDEV. (D or U)	CN	K	AREA Mi ²	INITIAL LENGTH Feet	SLOPE %	Ti Min	TRAVEL LENGTH Feet	SLOPE %	V ₁	V ₂	Tt Min	Tc Min	Tc Check Min	T _{lag}	
										VELOCITY	VELOCITY				0.6Tc/60	RAINFALL
										FPS	FPS				Hours	INCHES
(1)	(2)	(3)	(4)	(5b)	(6)	(7)	(8)	(9)	(10)	(10a)	(10b)	(11)	(12)	(13)	(14)	(15)
SUB1	U	83.0	0.7056	0.0274	180	5.00	5.6	1132	5.39	3.4	6.8	4.0	9.5	N/A	0.095	2.04
SUB2	U	83.0	0.7056	0.0113	288	4.17	7.5	1909	4.24	3.0	6.1	6.6	14.1	N/A	0.141	2.04
SUB2A	U	83.0	0.7056	0.0187	437	4.58	8.9	1213	3.05	2.6	5.1	5.5	14.5	N/A	0.145	2.04
SUB2B	U	83.0	0.7056	0.0086	214	1.40	9.3	343	14.29	5.6	11.1	1.0	10.3	N/A	0.103	2.04
SUB2C	U	83.0	0.7056	0.0064	242	18.18	4.2	874	5.72	3.5	7.0	3.2	7.4	N/A	0.074	2.04
SUB3	U	83.0	0.7056	0.0101	463	0.43	20.2	311	18.01	6.3	12.5	0.8	21.0	N/A	0.210	2.04
SUB3A	U	83.0	0.7056	0.0042	157.75	0.38	12.3	257	1.58	1.9	3.7	2.3	14.6	N/A	0.146	2.04
SUB4	U	83.0	0.7056	0.0325	352	1.42	11.8	1974	3.04	2.6	5.1	8.0	19.9	N/A	0.199	2.04
SUB5	U	83.0	0.7056	0.0043	97	42.90	2.0	520	5.47	3.5	6.9	2.5	4.5	N/A	0.045	2.04
SUB5A	U	83.0	0.7056	0.0026	80	0.63	7.4	86	0.58	1.1	2.2	1.3	8.7	N/A	0.087	2.04
SUB6	U	83.0	0.7056	0.0209	489	2.25	12.0	1130	5.84	3.6	7.1	3.8	15.8	N/A	0.158	2.04
SUB7	U	83.0	0.7056	0.0090	334	13.17	5.5	896	5.47	3.5	6.9	3.4	8.9	N/A	0.089	2.04
SUB8	U	83.0	0.7056	0.0044	239	11.30	4.9	499	7.62	4.1	8.1	2.0	6.9	N/A	0.069	2.04
SUB9	U	83.0	0.7056	0.0065	125	6.40	4.3	571	12.08	5.1	10.2	1.7	6.0	N/A	0.060	2.04
SUB10	U	83.0	0.7056	0.0051	160	9.38	4.3	838	8.59	4.3	8.6	2.6	6.8	N/A	0.068	2.04
SUB11	U	83.0	0.7056	0.0186	435	0.22	24.5	2550	5.14	3.4	6.7	7.6	32.1	N/A	0.321	2.04
SUB12	U	83.0	0.7056	0.0030	104.33	1.44	6.4	602	3.41	2.7	5.4	3.4	9.8	N/A	0.098	2.04
SUB12A	U	83.0	0.7056	0.0050	253.1	13.04	4.8	197	3.56	2.8	5.5	1.2	6.0	N/A	0.060	2.04
SLLE	U	83.0	0.7056	0.0284	144	22.22	3.0	2742	1.71	1.9	3.8	14.0	17.1	N/A	0.171	2.04

NOTE:

(1) Subbasin Name

(2) Developed or Undeveloped Subbasin

(3) Curve Number (See Subbasin CN Calculations)

(4) K = 0.0132 (CN) - 0.39

(5a) & (5b) Area

(6) Initial Length

(7) Initial Slope

(8) $T_i = 1.8 (1.1 - K) L^{1/2} / S^{1/3}$

(9) Travel Length

(10) Slope

(10a) Slope V_1 applies to the first 500 feet of travel distance;

Developed $V_1 = 20.2(S/100)^{1/2}$

(10b) V_2 applies to the remaining travel distance;

Developed $V_2 = 30.6(S/100)^{1/2}$

(11) $T_t = 500/(V_1*60) + (Travel\ Length - 500)/(V_2*60)$

(12) $T_c = T_i + T_t$

(13) Tc Check = L/180+10 (select smaller Tc)

(14) $T_{lag} = 0.6 T_c/60$

(15) Rainfall in inches

REFERENCE: Calculations based on the Clark County Regional Flood Control District HCDDM

STANDARD FORM 4

HEC-1 Computation 2

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
*
* RUN DATE 07OCT21 TIME 08:31:46 *
*

*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*

```

X   X   XXXXXX   XXXX   X
X   X   X   X   X   XX
X   X   X   X   X   X
XXXXXX XXXX   X   XXXXX X
X   X   X   X   X   X
X   X   X   X   X   X
X   X   XXXXXX   XXXX   XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM

*** FREE ***

```

1 ID RGS LANDFILL
2 ID AS-BUILT CONDITION
3 ID CCR DRAINAGE IMPROVEMENTS
4 ID INPUT FILE = RGSCCR25.DAT
5 ID INPUT FILE DATE = SEPT 2021
6 ID DESIGN STORM = 25-YEAR 24-HOUR STORM
7 ID NOAA ATLAS 14, VOLUME 1, VERSION 5
8 ID STORM DISTRIBUTION = NRCS 24-HOUR TYPE II
9 ID MODELED BY = JACOBS - FERNANDO ARAGONEZ
10 ID CHECKED BY = JACOBS - STEVE JONES, P.E.
11 ID
12 ID JR CARDS CONTAIN DARFS BASED ON THE FOLLOWING VALUES:
13 ID
14 ID AREA (SQUARE MILES) DARF
15 ID 0 ----- 1.000
16 ID
17 ID
18 IT 6 0 0 300
19 IO 6 0 0
20 IN 6 0 0
21 JR PREC 1.000
*

22 KK SUB1
23 BA 0.0274
24 PB 2.04
25 PC 0.00000 0.00101 0.00202 0.00305 0.00408 0.00513 0.00618 0.00725 0.00832 0.009
26 PC 0.01050 0.01161 0.01272 0.01385 0.01498 0.01613 0.01728 0.01845 0.01962 0.020
27 PC 0.02200 0.02321 0.02442 0.02565 0.02688 0.02813 0.02938 0.03065 0.03192 0.033
28 PC 0.03450 0.03581 0.03712 0.03845 0.03978 0.04113 0.04248 0.04385 0.04522 0.046
29 PC 0.04800 0.04941 0.05084 0.05229 0.05376 0.05525 0.05676 0.05829 0.05984 0.061
30 PC 0.06300 0.06461 0.06624 0.06956 0.07125 0.07296 0.07469 0.07644 0.07821 0.080
31 PC 0.08181 0.08364 0.08549 0.08736 0.08925 0.09116 0.09309 0.09504 0.09701 0.099
32 PC 0.10101 0.10304 0.10509 0.10716 0.10925 0.11136 0.11349 0.11564 0.11781 0.120
33 PC 0.12225 0.12460 0.12705 0.12960 0.13225 0.13785 0.14080 0.14385 0.14700 0.150
34 PC 0.15660 0.15980 0.16300 0.16628 0.16972 0.17332 0.17708 0.18100 0.18512 0.189
35 PC 0.19408 0.19892 0.20400 0.20940 0.21520 0.22140 0.22800 0.23500 0.24268 0.251
36 PC 0.26092 0.27148 0.28300 0.30684 0.35436 0.43079 0.56786 0.66300 0.68196 0.698
37 PC 0.71304 0.72516 0.73500 0.74344 0.75136 0.75876 0.76564 0.77200 0.77796 0.783
38 PC 0.78904 0.79416 0.79900 0.80360 0.80800 0.81220 0.81620 0.82000 0.82367 0.827
39 PC 0.83079 0.83424 0.83763 0.84094 0.84419 0.84736 0.85047 0.85350 0.85647 0.859
40 PC 0.86219 0.86494 0.86763 0.87024 0.87279 0.87526 0.87767 0.88000 0.88229 0.884
41 PC 0.88679 0.88900 0.89119 0.89335 0.89549 0.89760 0.89969 0.90175 0.90379 0.905
42 PC 0.90779 0.90975 0.91169 0.91360 0.91549 0.91735 0.91919 0.92100 0.92279 0.924
43 PC 0.92969 0.93135 0.93299 0.93460 0.93619 0.93775 0.93929 0.94080 0.94229 0.943
44 PC 0.94519 0.94660 0.94799 0.94935 0.95069 0.95200 0.95330 0.95459 0.95588 0.957
45 PC 0.95844 0.95971 0.96098 0.96224 0.96350 0.96475 0.96600 0.96724 0.96848 0.969
46 PC 0.97094 0.97216 0.97338 0.97459 0.97580 0.97700 0.97820 0.97939 0.98058 0.981
47 PC 0.98294 0.98411 0.98528 0.98644 0.98760 0.98875 0.98990 0.99104 0.99218 0.993
48 PC 0.99444 0.99556 0.99668 0.99779 0.99890 1.00000
49 LS 0 83.0
50 UD 0.095
*
```


HEC-1 INPUT

LINE	ID	1.....2.....3.....4.....5.....6.....7.....8.....9.....10
51	KK	RSUB1
52	KM	ROUTE SUB1 TO CWPS1
53	KM	LINING = EARTHEN
54	RD	797 0.018 0.025 0 TRAP 0 3
	*	
55	KK	SUB2
56	BA	0.0113
57	LS	0 83.0
58	UD	0.141
	*	
59	KK	CWPS1
60	KM	COMBINE SUB1, SUB2
61	KM	FLOW AT FIELD INLET
62	KM	COMBINATION POINT
63	HC	2
64	KO	3
	*	
65	KK	SLLE
66	BA	0.0284
67	LS	0 83.0
68	UD	0.171
	*	
69	KK	RSLLE
70	KM	ROUTE SLLE TO CWPS
71	KM	LINING = EARTHEN
72	RD	795 0.0400 0.013 0 CIRC 3
	*	
73	KK	CWPS
74	KM	COMBINE CWPS1 AND SLLE
75	KM	FLOW CWPS
76	KM	COMBINATION POINT
77	HC	2
78	KO	3
	*	
79	KK	RCWPS
80	KM	DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME
81	KO	1
82	RS	1 STOR -1
83	SV	0 5000
84	SE	0 5000
85	SQ	0 1
	*	

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
86	KK	SUB2A									
87	BA	0.0187									
88	LS	0	83.0								
89	UD	0.145									
	*										
90	KK	RSUB2A									
91	KM	ROUTE SUB2A TO SWB1									
92	KM	LINING = EARTHEN									
93	RD	507	0.093	0.033	0	TRAP	0	3			
	*										
94	KK	SUB2B									
95	BA	0.0086									
96	LS	0	83.0								
97	UD	0.103									
	*										
98	KK	SUB2C									
99	BA	0.0064									
100	LS	0	83.0								
101	UD	0.074									
	*										
102	KK	RSUB2C									
103	KM	ROUTE SUB2A TO SWB1									
104	KM	LINING = EARTHEN									
105	RD	328	0.030	0.025	0	TRAP	0	3			
	*										
106	KK	SUB3									
107	BA	0.0101									
108	LS	0	83.0								
109	UD	0.210									
	*										

```

110      KK      SWB1
111      KM      COMBINE SUB2A, SUB2B, SUB2C, AND SUB3
112      KM      FLOW SWB1
113      KM      COMBINATION POINT
114      HC        4
115      KO        3
      *

116      KK      RSWB1
117      KM      DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME
118      KO        1
119      RS        1      STOR      -1
120      SV        0      5000
121      SE        0      5000
122      SQ        0        1
      *

```

1

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

123      KK      SUB4
124      BA      0.0325
125      LS        0      83.0
126      UD      0.199
      *

127      KK      RSUB4
128      KM      ROUTE SUB4 TO SWB4
129      KM      LINING = EARTHEN
130      RD      118      0.017      0.025      0      TRAP      0      3
      *

131      KK      SUB5
132      BA      0.0043
133      LS        0      83.0
134      UD      0.045
      *

135      KK      RSUB5
136      KM      ROUTE SUB5 TO SWB3
137      KM      LINING = EARTHEN
138      RD      133      0.015      0.025      0      TRAP      0      3
      *

139      KK      SUB5A
140      BA      0.0026
141      LS        0      83.0
142      UD      0.087
      *

143      KK      SWB4
144      KM      COMBINE SUB4, SUB5, AND SUB5A
145      KM      FLOW SWB4
146      KM      COMBINATION POINT
147      HC        3
148      KO        3
      *

149      KK      RSWB4
150      KM      DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME
151      KO        1
152      RS        1      STOR      -1
153      SV        0      5000
154      SE        0      5000
155      SQ        0        1
      *

156      KK      SUB6
157      BA      0.0209
158      LS        0      83.0
159      UD      0.158
      *

```

1

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

160      KK      SUB7
161      BA      0.0090
162      LS        0      83.0
163      UD      0.089
      *

164      KK      RSUB7
165      KM      ROUTE SUB7 TO SWB5
166      KM      LINING = EARTHEN
167      RD      583      0.015      0.025      0      TRAP      0      3
      *

168      KK      SWB5
169      KM      COMBINE SUB6 AND SUB7

```



```

170      KM FLOW SWB5
171      KM COMBINATION POINT
172      HC      2
173      KO      3
174      *
175      KK      RSWB5
176      KM      DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME
177      KO      1
178      RS      1      STOR      -1
179      SV      0      5000
180      SE      0      5000
181      SQ      0      1
182      *
183      KK      SUB8
184      BA      0.0044
185      LS      0      83.0
186      UD      0.069
187      *
188      KK      RSWB8
189      KM      ROUTE SUB8 TO SWB8
190      KM      LINING = EARTHEN
191      RD      580      0.023      0.014      0      TRAP      0      2
192      *
193      KK      SUB9
194      BA      0.0065
195      LS      0      83.0
196      UD      0.060
197      *
198      KK      RSWB9
199      KM      ROUTE SUB9 TO SWB8
200      KM      LINING = EARTHEN
201      RD      118      0.034      0.025      0      TRAP      0      3
202      *

```

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

197      KK      SUB10
198      BA      0.0051
199      LS      0      83.0
200      UD      0.068
201      *
202      KK      SWB8
203      KM      COMBINE SUB8, SUB9 AND SUB10
204      KM      FLOW SWB8
205      KM      COMBINATION POINT
206      HC      3
207      KO      3
208      *
209      KK      RSWB8
210      KM      DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME
211      KO      1
212      RS      1      STOR      -1
213      SV      0      5000
214      SE      0      5000
215      SQ      0      1
216      *
217      KK      SUB12
218      KM      FLOW TO SWB6
219      BA      0.0030
220      LS      0      83.0
221      UD      0.098
222      *
223      KK      RSUB12
224      KM      DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME
225      KO      1
226      RS      1      STOR      -1
227      SV      0      5000
228      SE      0      5000
229      SQ      0      1
230      *
231      KK      SUB12A
232      KM      FLOW TO SWB7
233      BA      0.0052
234      LS      0      83.0
235      UD      0.060
236      *
237      KK      RSUB12A
238      KM      DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME
239      KO      1
240      RS      1      STOR      -1

```

235	SV	0	5000
236	SE	0	5000
237	SQ	0	1
	*		

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

238	KK	SUB3A	
239	KM	FLOW TO SWB3	
240	BA	0.0042	
241	LS	0	83.0
242	UD	0.146	
	*		

243	KK	RSUB3A	
244	KM	DUMMY STORAGE ROUTING TO COMPUTE ACCURATE STORAGE VOLUME	
245	KO	1	
246	RS	1	STOR -1
247	SV	0	5000
248	SE	0	5000
249	SQ	0	1
	*		

250	KK	SUB11	
251	KM	FLOW TO STORMWATER PONDS	
252	BA	0.0186	
253	LS	0	83.0
254	UD	0.321	
	*		
255	ZZ		

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(-->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
22	SUB1	
	V	
	V	
51	RSUB1	
	.	
55	.	SUB2
	.	.
	.	.
59	CWPS1.....	
	.	
	.	
65	.	SLLE
	.	V
	.	V
69	.	RSLE
	.	.
	.	.
73	CWPS.....	
	V	
	V	
79	RCWPS	
	.	
	.	
86	.	SUB2A
	.	V
	.	V
90	.	RSUB2A
	.	.
	.	.
94	.	SUB2B
	.	.
	.	.
98	.	.
	.	SUB2C
	.	V
	.	V
102	.	RSUB2C
	.	.
	.	.
106	.	.
	.	SUB3
	.	.
	.	.
110	.	SWB1.....
	V	
	V	
116	.	RSWB1
	.	.
	.	.
123	.	SUB4
	.	V
	.	V
127	.	RSUB4
	.	.
	.	.


```

131      .      .      .      SUB5
      .      .      .      V
      .      .      .      V
135      .      .      .      RSUB5
      .      .      .      .
139      .      .      .      SUB5A
      .      .      .      .
143      .      .      .      SWB4.....
      .      .      .      V
      .      .      .      V
149      .      .      .      RSWB4
      .      .      .      .
156      .      .      .      SUB6
      .      .      .      .
160      .      .      .      SUB7
      .      .      .      V
      .      .      .      V
164      .      .      .      RSUB7
      .      .      .      .
168      .      .      .      SWB5.....
      .      .      .      V
      .      .      .      V
174      .      .      .      RSWB5
      .      .      .      .
181      .      .      .      SUB8
      .      .      .      V
      .      .      .      V
185      .      .      .      RSUB8
      .      .      .      .
189      .      .      .      SUB9
      .      .      .      V
      .      .      .      V
193      .      .      .      RSUB9
      .      .      .      .
197      .      .      .      SUB10
      .      .      .      .
201      .      .      .      SWB8.....
      .      .      .      V
      .      .      .      V
207      .      .      .      RSWB8
      .      .      .      .
214      .      .      .      SUB12
      .      .      .      V
      .      .      .      V
219      .      .      .      RSUB12
      .      .      .      .
226      .      .      .      SUB12A
      .      .      .      V
      .      .      .      V
231      .      .      .      RSUB12A
      .      .      .      .
238      .      .      .      SUB3A
      .      .      .      V
      .      .      .      V
243      .      .      .      RSUB3A
      .      .      .      .
250      .      .      .      SUB11

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(***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998                     *
*   VERSION 4.1                   *
* RUN DATE 07OCT21 TIME 08:31:46 *
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET             *
* DAVIS, CALIFORNIA 95616       *
* (916) 756-1104                 *
*
*****

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RGS LANDFILL
AS-BUILT CONDITION
CCR DRAINAGE IMPROVEMENTS
INPUT FILE = RGSCCR25.DAT
INPUT FILE DATE = SEPT 2021
DESIGN STORM = 25-YEAR 24-HOUR STORM
NOAA ATLAS 14, VOLUME 1, VERSION 5

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STORM DISTRIBUTION = NRCS 24-HOUR TYPE II
MODELED BY = JACOBS - FERNANDO ARAGONEZ
CHECKED BY = JACOBS - STEVE JONES, P.E.

JR CARDS CONTAIN DARFS BASED ON THE FOLLOWING VALUES:

AREA (SQUARE MILES) DARF
0 1.000

19 IO OUTPUT CONTROL VARIABLES
 IPRNT 6 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 6 MINUTES IN COMPUTATION INTERVAL
 IDATE 1 0 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 2 0 ENDING DATE
 NDTIME 0554 ENDING TIME
 ICENT 19 CENTURY MARK

 COMPUTATION INTERVAL .10 HOURS
 TOTAL TIME BASE 29.90 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION
 NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 1.00

64 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

63 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION CWPS1
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(CFS)				
+	20.	11.70	2.	1.	1.	1.
		(INCHES)	.588	.722	.722	.722
		(AC-FT)	1.	1.	1.	1.
		CUMULATIVE AREA =	.04	SQ MI		

78 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

77 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION CWPS
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(CFS)				
+	32.	11.70	4.	1.	1.	1.
		(INCHES)	.588	.723	.723	.723
		(AC-FT)	2.	3.	3.	3.

CUMULATIVE AREA = .07 SQ MI

81 KO OUTPUT CONTROL VARIABLES
 IPRNT 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

82 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

83 SV STORAGE .0 5000.0

84 SE ELEVATION .00 5000.00

85 SQ DISCHARGE 0. 1.

HYDROGRAPH AT STATION RCWPS
PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1		0000	1	0.	.0	.0	*	1		1000	101	0.	.0	.0	*	1		2000	201	0.	2.4	2.4
1		0006	2	0.	.0	.0	*	1		1006	102	0.	.0	.0	*	1		2006	202	0.	2.4	2.4
1		0012	3	0.	.0	.0	*	1		1012	103	0.	.0	.0	*	1		2012	203	0.	2.4	2.4
1		0018	4	0.	.0	.0	*	1		1018	104	0.	.0	.0	*	1		2018	204	0.	2.4	2.4
1		0024	5	0.	.0	.0	*	1		1024	105	0.	.0	.0	*	1		2024	205	0.	2.4	2.4
1		0030	6	0.	.0	.0	*	1		1030	106	0.	.0	.0	*	1		2030	206	0.	2.4	2.4
1		0036	7	0.	.0	.0	*	1		1036	107	0.	.0	.0	*	1		2036	207	0.	2.4	2.4
1		0042	8	0.	.0	.0	*	1		1042	108	0.	.0	.0	*	1		2042	208	0.	2.4	2.4
1		0048	9	0.	.0	.0	*	1		1048	109	0.	.0	.0	*	1		2048	209	0.	2.4	2.4
1		0054	10	0.	.0	.0	*	1		1054	110	0.	.0	.0	*	1		2054	210	0.	2.4	2.4
1		0100	11	0.	.0	.0	*	1		1100	111	0.	.0	.0	*	1		2100	211	0.	2.4	2.4
1		0106	12	0.	.0	.0	*	1		1106	112	0.	.0	.0	*	1		2106	212	0.	2.4	2.4
1		0112	13	0.	.0	.0	*	1		1112	113	0.	.0	.0	*	1		2112	213	0.	2.4	2.4
1		0118	14	0.	.0	.0	*	1		1118	114	0.	.0	.0	*	1		2118	214	0.	2.4	2.4
1		0124	15	0.	.0	.0	*	1		1124	115	0.	.1	.1	*	1		2124	215	0.	2.5	2.5
1		0130	16	0.	.0	.0	*	1		1130	116	0.	.1	.1	*	1		2130	216	0.	2.5	2.5
1		0136	17	0.	.0	.0	*	1		1136	117	0.	.2	.2	*	1		2136	217	0.	2.5	2.5
1		0142	18	0.	.0	.0	*	1		1142	118	0.	.4	.4	*	1		2142	218	0.	2.5	2.5
1		0148	19	0.	.0	.0	*	1		1148	119	0.	.7	.7	*	1		2148	219	0.	2.5	2.5
1		0154	20	0.	.0	.0	*	1		1154	120	0.	.9	.9	*	1		2154	220	0.	2.5	2.5
1		0200	21	0.	.0	.0	*	1		1200	121	0.	1.1	1.1	*	1		2200	221	0.	2.5	2.5
1		0206	22	0.	.0	.0	*	1		1206	122	0.	1.2	1.2	*	1		2206	222	0.	2.5	2.5
1		0212	23	0.	.0	.0	*	1		1212	123	0.	1.2	1.2	*	1		2212	223	0.	2.5	2.5
1		0218	24	0.	.0	.0	*	1		1218	124	0.	1.3	1.3	*	1		2218	224	0.	2.5	2.5
1		0224	25	0.	.0	.0	*	1		1224	125	0.	1.3	1.3	*	1		2224	225	0.	2.5	2.5
1		0230	26	0.	.0	.0	*	1		1230	126	0.	1.4	1.4	*	1		2230	226	0.	2.5	2.5
1		0236	27	0.	.0	.0	*	1		1236	127	0.	1.4	1.4	*	1		2236	227	0.	2.5	2.5
1		0242	28	0.	.0	.0	*	1		1242	128	0.	1.4	1.4	*	1		2242	228	0.	2.5	2.5
1		0248	29	0.	.0	.0	*	1		1248	129	0.	1.5	1.5	*	1		2248	229	0.	2.5	2.5
1		0254	30	0.	.0	.0	*	1		1254	130	0.	1.5	1.5	*	1		2254	230	0.	2.5	2.5
1		0300	31	0.	.0	.0	*	1		1300	131	0.	1.5	1.5	*	1		2300	231	0.	2.5	2.5
1		0306	32	0.	.0	.0	*	1		1306	132	0.	1.6	1.6	*	1		2306	232	0.	2.5	2.5
1		0312	33	0.	.0	.0	*	1		1312	133	0.	1.6	1.6	*	1		2312	233	0.	2.6	2.6
1		0318	34	0.	.0	.0	*	1		1318	134	0.	1.6	1.6	*	1		2318	234	0.	2.6	2.6
1		0324	35	0.	.0	.0	*	1		1324	135	0.	1.6	1.6	*	1		2324	235	0.	2.6	2.6
1		0330	36	0.	.0	.0	*	1		1330	136	0.	1.6	1.6	*	1		2330	236	0.	2.6	2.6
1		0336	37	0.	.0	.0	*	1		1336	137	0.	1.7	1.7	*	1		2336	237	0.	2.6	2.6
1		0342	38	0.	.0	.0	*	1		1342	138	0.	1.7	1.7	*	1		2342	238	0.	2.6	2.6
1		0348	39	0.	.0	.0	*	1		1348	139	0.	1.7	1.7	*	1		2348	239	0.	2.6	2.6
1		0354	40	0.	.0	.0	*	1		1354	140	0.	1.7	1.7	*	1		2354	240	0.	2.6	2.6
1		0400	41	0.	.0	.0	*	1		1400	141	0.	1.7	1.7	*	2		0000	241	0.	2.6	2.6
1		0406	42	0.	.0	.0	*	1		1406	142	0.	1.8	1.8	*	2		0006	242	0.	2.6	2.6
1		0412	43	0.	.0	.0	*	1		1412	143	0.	1.8	1.8	*	2		0012	243	0.	2.6	2.6
1		0418	44	0.	.0	.0	*	1		1418	144	0.	1.8	1.8	*	2		0018	244	0.	2.6	2.6
1		0424	45	0.	.0	.0	*	1		1424	145	0.	1.8	1.8	*	2		0024	245	0.	2.6	2.6
1		0430	46	0.	.0	.0	*	1		1430	146	0.	1.8	1.8	*	2		0030	246	0.	2.6	2.6
1		0436	47	0.	.0	.0	*	1		1436	147	0.	1.8	1.8	*	2		0036	247	0.	2.6	2.6
1		0442	48	0.	.0	.0	*	1		1442	148	0.	1.8	1.8	*	2		0042	248	0.	2.6	2.6
1		0448	49	0.	.0	.0	*	1		1448	149	0.	1.9	1.9	*	2		0048	249	0.	2.6	2.6
1		0454	50	0.	.0	.0	*	1		1454	150	0.	1.9	1.9	*	2		0054	250	0.	2.6	2.6
1		0500	51	0.	.0	.0	*	1		1500	151	0.	1.9	1.9	*	2		0100	251	0.	2.6	2.6
1		0506	52	0.	.0	.0	*	1		1506	152	0.	1.9	1.9	*	2		0106	252	0.	2.6	2.6
1		0512	53	0.	.0	.0	*	1		1512	153	0.	1.9	1.9	*	2		0112	253	0.	2.6	2.6
1		0518	54	0.	.0	.0	*	1		1518	154	0.	1.9	1.9	*	2		0118	254	0.	2.6	2.6
1		0524	55	0.	.0	.0	*	1		1524	155	0.	1.9	1.9	*	2		0124	255	0.	2.6	2.6
1		0530	56	0.	.0	.0	*	1		1530	156	0.	2.0	2.0	*	2		0130	256	0.	2.6	2.6
1		0536	57	0.	.0	.0	*	1		1536	157	0.	2.0	2.0	*	2		0136	257	0.	2.6	2.6
1		0542	58	0.	.0	.0	*	1		1542	158	0.	2.0	2.0	*	2		0142	258	0.	2.6	2.6
1		0548	59	0.	.0	.0	*	1		1548	159	0.	2.0	2.0	*	2		0148	259	0.	2.6	2.6
1		0554	60	0.	.0	.0	*	1		1554	160	0.	2.0	2.0	*	2		0154	260	0.	2.6	2.6

1	0600	61	0.	.0	.0 * 1	1600 161	0.	2.0	2.0 * 2	0200 261	0.	2.6	2.6
1	0606	62	0.	.0	.0 * 1	1606 162	0.	2.0	2.0 * 2	0206 262	0.	2.6	2.6
1	0612	63	0.	.0	.0 * 1	1612 163	0.	2.0	2.0 * 2	0212 263	0.	2.6	2.6
1	0618	64	0.	.0	.0 * 1	1618 164	0.	2.0	2.0 * 2	0218 264	0.	2.6	2.6
1	0624	65	0.	.0	.0 * 1	1624 165	0.	2.1	2.1 * 2	0224 265	0.	2.6	2.6
1	0630	66	0.	.0	.0 * 1	1630 166	0.	2.1	2.1 * 2	0230 266	0.	2.6	2.6
1	0636	67	0.	.0	.0 * 1	1636 167	0.	2.1	2.1 * 2	0236 267	0.	2.6	2.6
1	0642	68	0.	.0	.0 * 1	1642 168	0.	2.1	2.1 * 2	0242 268	0.	2.6	2.6
1	0648	69	0.	.0	.0 * 1	1648 169	0.	2.1	2.1 * 2	0248 269	0.	2.6	2.6
1	0654	70	0.	.0	.0 * 1	1654 170	0.	2.1	2.1 * 2	0254 270	0.	2.6	2.6
1	0700	71	0.	.0	.0 * 1	1700 171	0.	2.1	2.1 * 2	0300 271	0.	2.6	2.6
1	0706	72	0.	.0	.0 * 1	1706 172	0.	2.1	2.1 * 2	0306 272	0.	2.6	2.6
1	0712	73	0.	.0	.0 * 1	1712 173	0.	2.1	2.1 * 2	0312 273	0.	2.6	2.6
1	0718	74	0.	.0	.0 * 1	1718 174	0.	2.1	2.1 * 2	0318 274	0.	2.6	2.6
1	0724	75	0.	.0	.0 * 1	1724 175	0.	2.1	2.1 * 2	0324 275	0.	2.6	2.6
1	0730	76	0.	.0	.0 * 1	1730 176	0.	2.2	2.2 * 2	0330 276	0.	2.6	2.6
1	0736	77	0.	.0	.0 * 1	1736 177	0.	2.2	2.2 * 2	0336 277	0.	2.6	2.6
1	0742	78	0.	.0	.0 * 1	1742 178	0.	2.2	2.2 * 2	0342 278	0.	2.6	2.6
1	0748	79	0.	.0	.0 * 1	1748 179	0.	2.2	2.2 * 2	0348 279	0.	2.6	2.6
1	0754	80	0.	.0	.0 * 1	1754 180	0.	2.2	2.2 * 2	0354 280	0.	2.6	2.6
1	0800	81	0.	.0	.0 * 1	1800 181	0.	2.2	2.2 * 2	0400 281	0.	2.6	2.6
1	0806	82	0.	.0	.0 * 1	1806 182	0.	2.2	2.2 * 2	0406 282	0.	2.6	2.6
1	0812	83	0.	.0	.0 * 1	1812 183	0.	2.2	2.2 * 2	0412 283	0.	2.6	2.6
1	0818	84	0.	.0	.0 * 1	1818 184	0.	2.2	2.2 * 2	0418 284	0.	2.6	2.6
1	0824	85	0.	.0	.0 * 1	1824 185	0.	2.3	2.3 * 2	0424 285	0.	2.6	2.6
1	0830	86	0.	.0	.0 * 1	1830 186	0.	2.3	2.3 * 2	0430 286	0.	2.6	2.6
1	0836	87	0.	.0	.0 * 1	1836 187	0.	2.3	2.3 * 2	0436 287	0.	2.6	2.6
1	0842	88	0.	.0	.0 * 1	1842 188	0.	2.3	2.3 * 2	0442 288	0.	2.6	2.6
1	0848	89	0.	.0	.0 * 1	1848 189	0.	2.3	2.3 * 2	0448 289	0.	2.6	2.6
1	0854	90	0.	.0	.0 * 1	1854 190	0.	2.3	2.3 * 2	0454 290	0.	2.6	2.6
1	0900	91	0.	.0	.0 * 1	1900 191	0.	2.3	2.3 * 2	0500 291	0.	2.6	2.6
1	0906	92	0.	.0	.0 * 1	1906 192	0.	2.3	2.3 * 2	0506 292	0.	2.6	2.6
1	0912	93	0.	.0	.0 * 1	1912 193	0.	2.3	2.3 * 2	0512 293	0.	2.6	2.6
1	0918	94	0.	.0	.0 * 1	1918 194	0.	2.3	2.3 * 2	0518 294	0.	2.6	2.6
1	0924	95	0.	.0	.0 * 1	1924 195	0.	2.3	2.3 * 2	0524 295	0.	2.6	2.6
1	0930	96	0.	.0	.0 * 1	1930 196	0.	2.3	2.3 * 2	0530 296	0.	2.6	2.6
1	0936	97	0.	.0	.0 * 1	1936 197	0.	2.3	2.3 * 2	0536 297	0.	2.6	2.6
1	0942	98	0.	.0	.0 * 1	1942 198	0.	2.3	2.3 * 2	0542 298	0.	2.6	2.6
1	0948	99	0.	.0	.0 * 1	1948 199	0.	2.4	2.4 * 2	0548 299	0.	2.6	2.6
1	0954	100	0.	.0	.0 * 1	1954 200	0.	2.4	2.4 * 2	0554 300	0.	2.6	2.6

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+	(CFS)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	0.	.00	0.	0.	0.	0.
		(INCHES)	.000	.000	.000	.000
		(AC-FT)	0.	0.	0.	0.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	3.	24.00	3.	2.	1.	1.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+	(FEET)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	2.59	24.40	2.59	1.71	1.37	1.37

CUMULATIVE AREA = .07 SQ MI

115 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

114 HC HYDROGRAPH COMBINATION
 ICOMP 4 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION SWB1
 FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+	(CFS)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	21.	11.70	3.	1.	1.	1.
		(CFS)	.589	.723	.723	.723
		(INCHES)	1.	2.	2.	2.
		(AC-FT)				

CUMULATIVE AREA = .04 SQ MI

118 KO OUTPUT CONTROL VARIABLES
 IPRNT 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL

QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

119 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

120 SV STORAGE .0 5000.0

121 SE ELEVATION .00 5000.00

122 SQ DISCHARGE 0. 1.

HYDROGRAPH AT STATION RSWB1
 PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1	0000	1	0.	.0	.0	.0	1	1000	101	0.	.0	.0	.0	1	2000	201	0.	1.5	1.5	
1	0006	2	0.	.0	.0	.0	1	1006	102	0.	.0	.0	.0	1	2006	202	0.	1.5	1.5	
1	0012	3	0.	.0	.0	.0	1	1012	103	0.	.0	.0	.0	1	2012	203	0.	1.6	1.6	
1	0018	4	0.	.0	.0	.0	1	1018	104	0.	.0	.0	.0	1	2018	204	0.	1.6	1.6	
1	0024	5	0.	.0	.0	.0	1	1024	105	0.	.0	.0	.0	1	2024	205	0.	1.6	1.6	
1	0030	6	0.	.0	.0	.0	1	1030	106	0.	.0	.0	.0	1	2030	206	0.	1.6	1.6	
1	0036	7	0.	.0	.0	.0	1	1036	107	0.	.0	.0	.0	1	2036	207	0.	1.6	1.6	
1	0042	8	0.	.0	.0	.0	1	1042	108	0.	.0	.0	.0	1	2042	208	0.	1.6	1.6	
1	0048	9	0.	.0	.0	.0	1	1048	109	0.	.0	.0	.0	1	2048	209	0.	1.6	1.6	
1	0054	10	0.	.0	.0	.0	1	1054	110	0.	.0	.0	.0	1	2054	210	0.	1.6	1.6	
1	0100	11	0.	.0	.0	.0	1	1100	111	0.	.0	.0	.0	1	2100	211	0.	1.6	1.6	
1	0106	12	0.	.0	.0	.0	1	1106	112	0.	.0	.0	.0	1	2106	212	0.	1.6	1.6	
1	0112	13	0.	.0	.0	.0	1	1112	113	0.	.0	.0	.0	1	2112	213	0.	1.6	1.6	
1	0118	14	0.	.0	.0	.0	1	1118	114	0.	.0	.0	.0	1	2118	214	0.	1.6	1.6	
1	0124	15	0.	.0	.0	.0	1	1124	115	0.	.0	.0	.0	1	2124	215	0.	1.6	1.6	
1	0130	16	0.	.0	.0	.0	1	1130	116	0.	.1	.1	.1	1	2130	216	0.	1.6	1.6	
1	0136	17	0.	.0	.0	.0	1	1136	117	0.	.1	.1	.1	1	2136	217	0.	1.6	1.6	
1	0142	18	0.	.0	.0	.0	1	1142	118	0.	.3	.3	.3	1	2142	218	0.	1.6	1.6	
1	0148	19	0.	.0	.0	.0	1	1148	119	0.	.5	.5	.5	1	2148	219	0.	1.6	1.6	
1	0154	20	0.	.0	.0	.0	1	1154	120	0.	.6	.6	.6	1	2154	220	0.	1.6	1.6	
1	0200	21	0.	.0	.0	.0	1	1200	121	0.	.7	.7	.7	1	2200	221	0.	1.6	1.6	
1	0206	22	0.	.0	.0	.0	1	1206	122	0.	.8	.8	.8	1	2206	222	0.	1.6	1.6	
1	0212	23	0.	.0	.0	.0	1	1212	123	0.	.8	.8	.8	1	2212	223	0.	1.6	1.6	
1	0218	24	0.	.0	.0	.0	1	1218	124	0.	.8	.8	.8	1	2218	224	0.	1.6	1.6	
1	0224	25	0.	.0	.0	.0	1	1224	125	0.	.9	.9	.9	1	2224	225	0.	1.6	1.6	
1	0230	26	0.	.0	.0	.0	1	1230	126	0.	.9	.9	.9	1	2230	226	0.	1.6	1.6	
1	0236	27	0.	.0	.0	.0	1	1236	127	0.	.9	.9	.9	1	2236	227	0.	1.6	1.6	
1	0242	28	0.	.0	.0	.0	1	1242	128	0.	.9	.9	.9	1	2242	228	0.	1.7	1.7	
1	0248	29	0.	.0	.0	.0	1	1248	129	0.	1.0	1.0	1.0	1	2248	229	0.	1.7	1.7	
1	0254	30	0.	.0	.0	.0	1	1254	130	0.	1.0	1.0	1.0	1	2254	230	0.	1.7	1.7	
1	0300	31	0.	.0	.0	.0	1	1300	131	0.	1.0	1.0	1.0	1	2300	231	0.	1.7	1.7	
1	0306	32	0.	.0	.0	.0	1	1306	132	0.	1.0	1.0	1.0	1	2306	232	0.	1.7	1.7	
1	0312	33	0.	.0	.0	.0	1	1312	133	0.	1.0	1.0	1.0	1	2312	233	0.	1.7	1.7	
1	0318	34	0.	.0	.0	.0	1	1318	134	0.	1.0	1.0	1.0	1	2318	234	0.	1.7	1.7	
1	0324	35	0.	.0	.0	.0	1	1324	135	0.	1.1	1.1	1.1	1	2324	235	0.	1.7	1.7	
1	0330	36	0.	.0	.0	.0	1	1330	136	0.	1.1	1.1	1.1	1	2330	236	0.	1.7	1.7	
1	0336	37	0.	.0	.0	.0	1	1336	137	0.	1.1	1.1	1.1	1	2336	237	0.	1.7	1.7	
1	0342	38	0.	.0	.0	.0	1	1342	138	0.	1.1	1.1	1.1	1	2342	238	0.	1.7	1.7	
1	0348	39	0.	.0	.0	.0	1	1348	139	0.	1.1	1.1	1.1	1	2348	239	0.	1.7	1.7	
1	0354	40	0.	.0	.0	.0	1	1354	140	0.	1.1	1.1	1.1	1	2354	240	0.	1.7	1.7	
1	0400	41	0.	.0	.0	.0	1	1400	141	0.	1.1	1.1	1.1	2	0000	241	0.	1.7	1.7	
1	0406	42	0.	.0	.0	.0	1	1406	142	0.	1.1	1.1	1.1	2	0006	242	0.	1.7	1.7	
1	0412	43	0.	.0	.0	.0	1	1412	143	0.	1.2	1.2	1.2	2	0012	243	0.	1.7	1.7	
1	0418	44	0.	.0	.0	.0	1	1418	144	0.	1.2	1.2	1.2	2	0018	244	0.	1.7	1.7	
1	0424	45	0.	.0	.0	.0	1	1424	145	0.	1.2	1.2	1.2	2	0024	245	0.	1.7	1.7	
1	0430	46	0.	.0	.0	.0	1	1430	146	0.	1.2	1.2	1.2	2	0030	246	0.	1.7	1.7	
1	0436	47	0.	.0	.0	.0	1	1436	147	0.	1.2	1.2	1.2	2	0036	247	0.	1.7	1.7	
1	0442	48	0.	.0	.0	.0	1	1442	148	0.	1.2	1.2	1.2	2	0042	248	0.	1.7	1.7	
1	0448	49	0.	.0	.0	.0	1	1448	149	0.	1.2	1.2	1.2	2	0048	249	0.	1.7	1.7	
1	0454	50	0.	.0	.0	.0	1	1454	150	0.	1.2	1.2	1.2	2	0054	250	0.	1.7	1.7	
1	0500	51	0.	.0	.0	.0	1	1500	151	0.	1.2	1.2	1.2	2	0100	251	0.	1.7	1.7	
1	0506	52	0.	.0	.0	.0	1	1506	152	0.	1.2	1.2	1.2	2	0106	252	0.	1.7	1.7	
1	0512	53	0.	.0	.0	.0	1	1512	153	0.	1.3	1.3	1.3	2	0112	253	0.	1.7	1.7	
1	0518	54	0.	.0	.0	.0	1	1518	154	0.	1.3	1.3	1.3	2	0118	254	0.	1.7	1.7	
1	0524	55	0.	.0	.0	.0	1	1524	155	0.	1.3	1.3	1.3	2	0124	255	0.	1.7	1.7	
1	0530	56	0.	.0	.0	.0	1	1530	156	0.	1.3	1.3	1.3	2	0130	256	0.	1.7	1.7	
1	0536	57	0.	.0	.0	.0	1	1536	157	0.	1.3	1.3	1.3	2	0136	257	0.	1.7	1.7	
1	0542	58	0.	.0	.0	.0	1	1542	158	0.	1.3	1.3	1.3	2	0142	258	0.	1.7	1.7	
1	0548	59	0.	.0	.0	.0	1	1548	159	0.	1.3	1.3	1.3	2	0148	259	0.	1.7	1.7	
1	0554	60	0.	.0	.0	.0	1	1554	160	0.	1.3	1.3	1.3	2	0154	260	0.	1.7	1.7	
1	0600	61	0.	.0	.0	.0	1	1600	161	0.	1.3	1.3	1.3	2	0200	261	0.	1.7	1.7	
1	0606	62	0.	.0	.0	.0	1	1606	162	0.	1.3	1.3	1.3	2	0206	262	0.	1.7	1.7	
1	0612	63	0.	.0	.0	.0	1	1612	163	0.	1.3	1.3	1.3	2	0212	263	0.	1.7	1.7	
1	0618	64	0.	.0	.0	.0	1	1618	164	0.	1.3	1.3	1.3	2	0218	264	0.	1.7	1.7	
1	0624	65	0.	.0	.0	.0	1	1624	165	0.	1.3	1.3	1.3	2	0224	265	0.	1.7	1.7	
1	0630	66	0.	.0	.0	.0	1	1630	166	0.	1.3	1.3	1.3	2	0230	266	0.	1.7	1.7	

1	0636	67	0.	.0	.0 * 1	1636 167	0.	1.4	1.4 * 2	0236 267	0.	1.7	1.7
1	0642	68	0.	.0	.0 * 1	1642 168	0.	1.4	1.4 * 2	0242 268	0.	1.7	1.7
1	0648	69	0.	.0	.0 * 1	1648 169	0.	1.4	1.4 * 2	0248 269	0.	1.7	1.7
1	0654	70	0.	.0	.0 * 1	1654 170	0.	1.4	1.4 * 2	0254 270	0.	1.7	1.7
1	0700	71	0.	.0	.0 * 1	1700 171	0.	1.4	1.4 * 2	0300 271	0.	1.7	1.7
1	0706	72	0.	.0	.0 * 1	1706 172	0.	1.4	1.4 * 2	0306 272	0.	1.7	1.7
1	0712	73	0.	.0	.0 * 1	1712 173	0.	1.4	1.4 * 2	0312 273	0.	1.7	1.7
1	0718	74	0.	.0	.0 * 1	1718 174	0.	1.4	1.4 * 2	0318 274	0.	1.7	1.7
1	0724	75	0.	.0	.0 * 1	1724 175	0.	1.4	1.4 * 2	0324 275	0.	1.7	1.7
1	0730	76	0.	.0	.0 * 1	1730 176	0.	1.4	1.4 * 2	0330 276	0.	1.7	1.7
1	0736	77	0.	.0	.0 * 1	1736 177	0.	1.4	1.4 * 2	0336 277	0.	1.7	1.7
1	0742	78	0.	.0	.0 * 1	1742 178	0.	1.4	1.4 * 2	0342 278	0.	1.7	1.7
1	0748	79	0.	.0	.0 * 1	1748 179	0.	1.4	1.4 * 2	0348 279	0.	1.7	1.7
1	0754	80	0.	.0	.0 * 1	1754 180	0.	1.4	1.4 * 2	0354 280	0.	1.7	1.7
1	0800	81	0.	.0	.0 * 1	1800 181	0.	1.4	1.4 * 2	0400 281	0.	1.7	1.7
1	0806	82	0.	.0	.0 * 1	1806 182	0.	1.4	1.4 * 2	0406 282	0.	1.7	1.7
1	0812	83	0.	.0	.0 * 1	1812 183	0.	1.5	1.5 * 2	0412 283	0.	1.7	1.7
1	0818	84	0.	.0	.0 * 1	1818 184	0.	1.5	1.5 * 2	0418 284	0.	1.7	1.7
1	0824	85	0.	.0	.0 * 1	1824 185	0.	1.5	1.5 * 2	0424 285	0.	1.7	1.7
1	0830	86	0.	.0	.0 * 1	1830 186	0.	1.5	1.5 * 2	0430 286	0.	1.7	1.7
1	0836	87	0.	.0	.0 * 1	1836 187	0.	1.5	1.5 * 2	0436 287	0.	1.7	1.7
1	0842	88	0.	.0	.0 * 1	1842 188	0.	1.5	1.5 * 2	0442 288	0.	1.7	1.7
1	0848	89	0.	.0	.0 * 1	1848 189	0.	1.5	1.5 * 2	0448 289	0.	1.7	1.7
1	0854	90	0.	.0	.0 * 1	1854 190	0.	1.5	1.5 * 2	0454 290	0.	1.7	1.7
1	0900	91	0.	.0	.0 * 1	1900 191	0.	1.5	1.5 * 2	0500 291	0.	1.7	1.7
1	0906	92	0.	.0	.0 * 1	1906 192	0.	1.5	1.5 * 2	0506 292	0.	1.7	1.7
1	0912	93	0.	.0	.0 * 1	1912 193	0.	1.5	1.5 * 2	0512 293	0.	1.7	1.7
1	0918	94	0.	.0	.0 * 1	1918 194	0.	1.5	1.5 * 2	0518 294	0.	1.7	1.7
1	0924	95	0.	.0	.0 * 1	1924 195	0.	1.5	1.5 * 2	0524 295	0.	1.7	1.7
1	0930	96	0.	.0	.0 * 1	1930 196	0.	1.5	1.5 * 2	0530 296	0.	1.7	1.7
1	0936	97	0.	.0	.0 * 1	1936 197	0.	1.5	1.5 * 2	0536 297	0.	1.7	1.7
1	0942	98	0.	.0	.0 * 1	1942 198	0.	1.5	1.5 * 2	0542 298	0.	1.7	1.7
1	0948	99	0.	.0	.0 * 1	1948 199	0.	1.5	1.5 * 2	0548 299	0.	1.7	1.7
1	0954	100	0.	.0	.0 * 1	1954 200	0.	1.5	1.5 * 2	0554 300	0.	1.7	1.7

PEAK FLOW	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(HR)				
+	0.	.00				
		(CFS)				
		(INCHES)	.000	.000	.000	.000
		(AC-FT)	0.	0.	0.	0.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(AC-FT)	(HR)				
	2.	24.10	2.	1.	1.	1.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(FEET)	(HR)				
	1.69	24.50	1.69	1.12	.90	.90

CUMULATIVE AREA = .04 SQ MI

148 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

147 HC HYDROGRAPH COMBINATION

ICOMP	3	NUMBER OF HYDROGRAPHS TO COMBINE
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HYDROGRAPH AT STATION SWB4
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(HR)				
+	17.	11.80				
		(CFS)				
		(INCHES)	.588	.723	.723	.723
		(AC-FT)	1.	2.	2.	2.

CUMULATIVE AREA = .04 SQ MI

151 KO OUTPUT CONTROL VARIABLES

IPRNT	1	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

152 RS STORAGE ROUTING

NSTPS	1	NUMBER OF SUBREACHES
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	ITYP	STOR	TYPE OF INITIAL CONDITION
	RSVRIC	-1.00	INITIAL CONDITION
	X	.00	WORKING R AND D COEFFICIENT
153 SV	STORAGE	.0	5000.0
154 SE	ELEVATION	.00	5000.00
155 SQ	DISCHARGE	0.	1.

HYDROGRAPH AT STATION RSWB4
PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1	0000	1	0.	.0	.0	.0	*	1	1000	101	0.	.0	.0	.0	*	1	2000	201	0.	1.4	1.4	
1	0006	2	0.	.0	.0	.0	*	1	1006	102	0.	.0	.0	.0	*	1	2006	202	0.	1.4	1.4	
1	0012	3	0.	.0	.0	.0	*	1	1012	103	0.	.0	.0	.0	*	1	2012	203	0.	1.4	1.4	
1	0018	4	0.	.0	.0	.0	*	1	1018	104	0.	.0	.0	.0	*	1	2018	204	0.	1.4	1.4	
1	0024	5	0.	.0	.0	.0	*	1	1024	105	0.	.0	.0	.0	*	1	2024	205	0.	1.4	1.4	
1	0030	6	0.	.0	.0	.0	*	1	1030	106	0.	.0	.0	.0	*	1	2030	206	0.	1.4	1.4	
1	0036	7	0.	.0	.0	.0	*	1	1036	107	0.	.0	.0	.0	*	1	2036	207	0.	1.4	1.4	
1	0042	8	0.	.0	.0	.0	*	1	1042	108	0.	.0	.0	.0	*	1	2042	208	0.	1.4	1.4	
1	0048	9	0.	.0	.0	.0	*	1	1048	109	0.	.0	.0	.0	*	1	2048	209	0.	1.4	1.4	
1	0054	10	0.	.0	.0	.0	*	1	1054	110	0.	.0	.0	.0	*	1	2054	210	0.	1.4	1.4	
1	0100	11	0.	.0	.0	.0	*	1	1100	111	0.	.0	.0	.0	*	1	2100	211	0.	1.4	1.4	
1	0106	12	0.	.0	.0	.0	*	1	1106	112	0.	.0	.0	.0	*	1	2106	212	0.	1.4	1.4	
1	0112	13	0.	.0	.0	.0	*	1	1112	113	0.	.0	.0	.0	*	1	2112	213	0.	1.4	1.4	
1	0118	14	0.	.0	.0	.0	*	1	1118	114	0.	.0	.0	.0	*	1	2118	214	0.	1.4	1.4	
1	0124	15	0.	.0	.0	.0	*	1	1124	115	0.	.0	.0	.0	*	1	2124	215	0.	1.4	1.4	
1	0130	16	0.	.0	.0	.0	*	1	1130	116	0.	.1	.1	.1	*	1	2130	216	0.	1.4	1.4	
1	0136	17	0.	.0	.0	.0	*	1	1136	117	0.	.1	.1	.1	*	1	2136	217	0.	1.4	1.4	
1	0142	18	0.	.0	.0	.0	*	1	1142	118	0.	.2	.2	.2	*	1	2142	218	0.	1.4	1.4	
1	0148	19	0.	.0	.0	.0	*	1	1148	119	0.	.4	.4	.4	*	1	2148	219	0.	1.5	1.5	
1	0154	20	0.	.0	.0	.0	*	1	1154	120	0.	.5	.5	.5	*	1	2154	220	0.	1.5	1.5	
1	0200	21	0.	.0	.0	.0	*	1	1200	121	0.	.6	.6	.6	*	1	2200	221	0.	1.5	1.5	
1	0206	22	0.	.0	.0	.0	*	1	1206	122	0.	.7	.7	.7	*	1	2206	222	0.	1.5	1.5	
1	0212	23	0.	.0	.0	.0	*	1	1212	123	0.	.7	.7	.7	*	1	2212	223	0.	1.5	1.5	
1	0218	24	0.	.0	.0	.0	*	1	1218	124	0.	.7	.7	.7	*	1	2218	224	0.	1.5	1.5	
1	0224	25	0.	.0	.0	.0	*	1	1224	125	0.	.8	.8	.8	*	1	2224	225	0.	1.5	1.5	
1	0230	26	0.	.0	.0	.0	*	1	1230	126	0.	.8	.8	.8	*	1	2230	226	0.	1.5	1.5	
1	0236	27	0.	.0	.0	.0	*	1	1236	127	0.	.8	.8	.8	*	1	2236	227	0.	1.5	1.5	
1	0242	28	0.	.0	.0	.0	*	1	1242	128	0.	.8	.8	.8	*	1	2242	228	0.	1.5	1.5	
1	0248	29	0.	.0	.0	.0	*	1	1248	129	0.	.9	.9	.9	*	1	2248	229	0.	1.5	1.5	
1	0254	30	0.	.0	.0	.0	*	1	1254	130	0.	.9	.9	.9	*	1	2254	230	0.	1.5	1.5	
1	0300	31	0.	.0	.0	.0	*	1	1300	131	0.	.9	.9	.9	*	1	2300	231	0.	1.5	1.5	
1	0306	32	0.	.0	.0	.0	*	1	1306	132	0.	.9	.9	.9	*	1	2306	232	0.	1.5	1.5	
1	0312	33	0.	.0	.0	.0	*	1	1312	133	0.	.9	.9	.9	*	1	2312	233	0.	1.5	1.5	
1	0318	34	0.	.0	.0	.0	*	1	1318	134	0.	.9	.9	.9	*	1	2318	234	0.	1.5	1.5	
1	0324	35	0.	.0	.0	.0	*	1	1324	135	0.	1.0	1.0	1.0	*	1	2324	235	0.	1.5	1.5	
1	0330	36	0.	.0	.0	.0	*	1	1330	136	0.	1.0	1.0	1.0	*	1	2330	236	0.	1.5	1.5	
1	0336	37	0.	.0	.0	.0	*	1	1336	137	0.	1.0	1.0	1.0	*	1	2336	237	0.	1.5	1.5	
1	0342	38	0.	.0	.0	.0	*	1	1342	138	0.	1.0	1.0	1.0	*	1	2342	238	0.	1.5	1.5	
1	0348	39	0.	.0	.0	.0	*	1	1348	139	0.	1.0	1.0	1.0	*	1	2348	239	0.	1.5	1.5	
1	0354	40	0.	.0	.0	.0	*	1	1354	140	0.	1.0	1.0	1.0	*	1	2354	240	0.	1.5	1.5	
1	0400	41	0.	.0	.0	.0	*	1	1400	141	0.	1.0	1.0	1.0	*	2	0000	241	0.	1.5	1.5	
1	0406	42	0.	.0	.0	.0	*	1	1406	142	0.	1.0	1.0	1.0	*	2	0006	242	0.	1.5	1.5	
1	0412	43	0.	.0	.0	.0	*	1	1412	143	0.	1.0	1.0	1.0	*	2	0012	243	0.	1.5	1.5	
1	0418	44	0.	.0	.0	.0	*	1	1418	144	0.	1.0	1.0	1.0	*	2	0018	244	0.	1.5	1.5	
1	0424	45	0.	.0	.0	.0	*	1	1424	145	0.	1.1	1.1	1.1	*	2	0024	245	0.	1.5	1.5	
1	0430	46	0.	.0	.0	.0	*	1	1430	146	0.	1.1	1.1	1.1	*	2	0030	246	0.	1.5	1.5	
1	0436	47	0.	.0	.0	.0	*	1	1436	147	0.	1.1	1.1	1.1	*	2	0036	247	0.	1.5	1.5	
1	0442	48	0.	.0	.0	.0	*	1	1442	148	0.	1.1	1.1	1.1	*	2	0042	248	0.	1.5	1.5	
1	0448	49	0.	.0	.0	.0	*	1	1448	149	0.	1.1	1.1	1.1	*	2	0048	249	0.	1.5	1.5	
1	0454	50	0.	.0	.0	.0	*	1	1454	150	0.	1.1	1.1	1.1	*	2	0054	250	0.	1.5	1.5	
1	0500	51	0.	.0	.0	.0	*	1	1500	151	0.	1.1	1.1	1.1	*	2	0100	251	0.	1.5	1.5	
1	0506	52	0.	.0	.0	.0	*	1	1506	152	0.	1.1	1.1	1.1	*	2	0106	252	0.	1.5	1.5	
1	0512	53	0.	.0	.0	.0	*	1	1512	153	0.	1.1	1.1	1.1	*	2	0112	253	0.	1.5	1.5	
1	0518	54	0.	.0	.0	.0	*	1	1518	154	0.	1.1	1.1	1.1	*	2	0118	254	0.	1.5	1.5	
1	0524	55	0.	.0	.0	.0	*	1	1524	155	0.	1.1	1.1	1.1	*	2	0124	255	0.	1.5	1.5	
1	0530	56	0.	.0	.0	.0	*	1	1530	156	0.	1.1	1.1	1.1	*	2	0130	256	0.	1.5	1.5	
1	0536	57	0.	.0	.0	.0	*	1	1536	157	0.	1.2	1.2	1.2	*	2	0136	257	0.	1.5	1.5	
1	0542	58	0.	.0	.0	.0	*	1	1542	158	0.	1.2	1.2	1.2	*	2	0142	258	0.	1.5	1.5	
1	0548	59	0.	.0	.0	.0	*	1	1548	159	0.	1.2	1.2	1.2	*	2	0148	259	0.	1.5	1.5	
1	0554	60	0.	.0	.0	.0	*	1	1554	160	0.	1.2	1.2	1.2	*	2	0154	260	0.	1.5	1.5	
1	0600	61	0.	.0	.0	.0	*	1	1600	161	0.	1.2	1.2	1.2	*	2	0200	261	0.	1.5	1.5	
1	0606	62	0.	.0	.0	.0	*	1	1606	162	0.	1.2	1.2	1.2	*	2	0206	262	0.	1.5	1.5	
1	0612	63	0.	.0	.0	.0	*	1	1612	163	0.	1.2	1.2	1.2	*	2	0212	263	0.	1.5	1.5	
1	0618	64	0.	.0	.0	.0	*	1	1618	164	0.	1.2	1.2	1.2	*	2	0218	264	0.	1.5	1.5	
1	0624	65	0.	.0	.0	.0	*	1	1624	165	0.	1.2	1.2	1.2	*	2	0224	265	0.	1.5	1.5	
1	0630	66	0.	.0	.0	.0	*	1	1630	166	0.	1.2	1.2	1.2	*	2	0230	266	0.	1.5	1.5	
1	0636	67	0.	.0	.0	.0	*	1	1636	167	0.	1.2	1.2	1.2	*	2	0236	267	0.	1.5	1.5	
1	0642	68	0.	.0	.0	.0	*	1	1642	168	0.	1.2	1.2	1.2	*	2	0242	268	0.	1.5	1.5	
1	0648	69	0.	.0	.0	.0	*	1	1648	169	0.	1.2	1.2	1.2	*	2	0248	269	0.	1.5	1.5	
1	0654	70	0.	.0	.0	.0	*	1	1654	170	0.	1.2	1.2	1.2	*	2	0254	270	0.	1.5	1.5	
1	0700	71	0.	.0	.0	.0	*	1	1700	171	0.	1.2	1.2	1.2	*	2	0300	271	0.	1.5	1.5	
1	0706	72	0.	.0	.0	.0	*	1	1706	172	0.	1.2	1.2	1.2	*	2	0306	272	0.	1.5	1.5	

1	0712	73	0.	.0	.0 * 1	1712 173	0.	1.2	1.2 * 2	0312 273	0.	1.5	1.5
1	0718	74	0.	.0	.0 * 1	1718 174	0.	1.3	1.3 * 2	0318 274	0.	1.5	1.5
1	0724	75	0.	.0	.0 * 1	1724 175	0.	1.3	1.3 * 2	0324 275	0.	1.5	1.5
1	0730	76	0.	.0	.0 * 1	1730 176	0.	1.3	1.3 * 2	0330 276	0.	1.5	1.5
1	0736	77	0.	.0	.0 * 1	1736 177	0.	1.3	1.3 * 2	0336 277	0.	1.5	1.5
1	0742	78	0.	.0	.0 * 1	1742 178	0.	1.3	1.3 * 2	0342 278	0.	1.5	1.5
1	0748	79	0.	.0	.0 * 1	1748 179	0.	1.3	1.3 * 2	0348 279	0.	1.5	1.5
1	0754	80	0.	.0	.0 * 1	1754 180	0.	1.3	1.3 * 2	0354 280	0.	1.5	1.5
1	0800	81	0.	.0	.0 * 1	1800 181	0.	1.3	1.3 * 2	0400 281	0.	1.5	1.5
1	0806	82	0.	.0	.0 * 1	1806 182	0.	1.3	1.3 * 2	0406 282	0.	1.5	1.5
1	0812	83	0.	.0	.0 * 1	1812 183	0.	1.3	1.3 * 2	0412 283	0.	1.5	1.5
1	0818	84	0.	.0	.0 * 1	1818 184	0.	1.3	1.3 * 2	0418 284	0.	1.5	1.5
1	0824	85	0.	.0	.0 * 1	1824 185	0.	1.3	1.3 * 2	0424 285	0.	1.5	1.5
1	0830	86	0.	.0	.0 * 1	1830 186	0.	1.3	1.3 * 2	0430 286	0.	1.5	1.5
1	0836	87	0.	.0	.0 * 1	1836 187	0.	1.3	1.3 * 2	0436 287	0.	1.5	1.5
1	0842	88	0.	.0	.0 * 1	1842 188	0.	1.3	1.3 * 2	0442 288	0.	1.5	1.5
1	0848	89	0.	.0	.0 * 1	1848 189	0.	1.3	1.3 * 2	0448 289	0.	1.5	1.5
1	0854	90	0.	.0	.0 * 1	1854 190	0.	1.3	1.3 * 2	0454 290	0.	1.5	1.5
1	0900	91	0.	.0	.0 * 1	1900 191	0.	1.3	1.3 * 2	0500 291	0.	1.5	1.5
1	0906	92	0.	.0	.0 * 1	1906 192	0.	1.4	1.4 * 2	0506 292	0.	1.5	1.5
1	0912	93	0.	.0	.0 * 1	1912 193	0.	1.4	1.4 * 2	0512 293	0.	1.5	1.5
1	0918	94	0.	.0	.0 * 1	1918 194	0.	1.4	1.4 * 2	0518 294	0.	1.5	1.5
1	0924	95	0.	.0	.0 * 1	1924 195	0.	1.4	1.4 * 2	0524 295	0.	1.5	1.5
1	0930	96	0.	.0	.0 * 1	1930 196	0.	1.4	1.4 * 2	0530 296	0.	1.5	1.5
1	0936	97	0.	.0	.0 * 1	1936 197	0.	1.4	1.4 * 2	0536 297	0.	1.5	1.5
1	0942	98	0.	.0	.0 * 1	1942 198	0.	1.4	1.4 * 2	0542 298	0.	1.5	1.5
1	0948	99	0.	.0	.0 * 1	1948 199	0.	1.4	1.4 * 2	0548 299	0.	1.5	1.5
1	0954	100	0.	.0	.0 * 1	1954 200	0.	1.4	1.4 * 2	0554 300	0.	1.5	1.5

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+	(CFS)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	0.	.00	(CFS)			
			(INCHES)	.000	.000	.000
			(AC-FT)	0.	0.	0.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	2.	24.00				
			2.	1.	1.	1.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+	(FEET)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	1.52	24.60				
			1.52	1.00	.81	.81

CUMULATIVE AREA = .04 SQ MI

173 K0 OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

172 HC HYDROGRAPH COMBINATION

ICOMP	2	NUMBER OF HYDROGRAPHS TO COMBINE
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HYDROGRAPH AT STATION SWB5
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+	(CFS)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	15.	11.70	(CFS)			
			(INCHES)	.588	.722	.722
			(AC-FT)	1.	1.	1.

CUMULATIVE AREA = .03 SQ MI

176 K0 OUTPUT CONTROL VARIABLES

IPRNT	1	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

177 RS STORAGE ROUTING

NSTPS	1	NUMBER OF SUBREACHES
ITYP	STOR	TYPE OF INITIAL CONDITION
RSVRIC	-1.00	INITIAL CONDITION
X	.00	WORKING R AND D COEFFICIENT

178 SV STORAGE .0 5000.0

179 SE ELEVATION .00 5000.00
180 SQ DISCHARGE 0. 1.

HYDROGRAPH AT STATION RSWB5
PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
							*								*							
1	0000	1	0.	0.	0.	0	*	1	1000	101	0.	0.	0.	0	*	1	2000	201	0.	1.1	1.1	
1	0006	2	0.	0.	0.	0	*	1	1006	102	0.	0.	0.	0	*	1	2006	202	0.	1.1	1.1	
1	0012	3	0.	0.	0.	0	*	1	1012	103	0.	0.	0.	0	*	1	2012	203	0.	1.1	1.1	
1	0018	4	0.	0.	0.	0	*	1	1018	104	0.	0.	0.	0	*	1	2018	204	0.	1.1	1.1	
1	0024	5	0.	0.	0.	0	*	1	1024	105	0.	0.	0.	0	*	1	2024	205	0.	1.1	1.1	
1	0030	6	0.	0.	0.	0	*	1	1030	106	0.	0.	0.	0	*	1	2030	206	0.	1.1	1.1	
1	0036	7	0.	0.	0.	0	*	1	1036	107	0.	0.	0.	0	*	1	2036	207	0.	1.1	1.1	
1	0042	8	0.	0.	0.	0	*	1	1042	108	0.	0.	0.	0	*	1	2042	208	0.	1.1	1.1	
1	0048	9	0.	0.	0.	0	*	1	1048	109	0.	0.	0.	0	*	1	2048	209	0.	1.1	1.1	
1	0054	10	0.	0.	0.	0	*	1	1054	110	0.	0.	0.	0	*	1	2054	210	0.	1.1	1.1	
1	0100	11	0.	0.	0.	0	*	1	1100	111	0.	0.	0.	0	*	1	2100	211	0.	1.1	1.1	
1	0106	12	0.	0.	0.	0	*	1	1106	112	0.	0.	0.	0	*	1	2106	212	0.	1.1	1.1	
1	0112	13	0.	0.	0.	0	*	1	1112	113	0.	0.	0.	0	*	1	2112	213	0.	1.1	1.1	
1	0118	14	0.	0.	0.	0	*	1	1118	114	0.	0.	0.	0	*	1	2118	214	0.	1.1	1.1	
1	0124	15	0.	0.	0.	0	*	1	1124	115	0.	0.	0.	0	*	1	2124	215	0.	1.1	1.1	
1	0130	16	0.	0.	0.	0	*	1	1130	116	0.	0.	0.	0	*	1	2130	216	0.	1.1	1.1	
1	0136	17	0.	0.	0.	0	*	1	1136	117	0.	1.	1.	1	*	1	2136	217	0.	1.1	1.1	
1	0142	18	0.	0.	0.	0	*	1	1142	118	0.	2.	2.	2	*	1	2142	218	0.	1.1	1.1	
1	0148	19	0.	0.	0.	0	*	1	1148	119	0.	3.	3.	3	*	1	2148	219	0.	1.1	1.1	
1	0154	20	0.	0.	0.	0	*	1	1154	120	0.	4.	4.	4	*	1	2154	220	0.	1.1	1.1	
1	0200	21	0.	0.	0.	0	*	1	1200	121	0.	5.	5.	5	*	1	2200	221	0.	1.1	1.1	
1	0206	22	0.	0.	0.	0	*	1	1206	122	0.	5.	5.	5	*	1	2206	222	0.	1.1	1.1	
1	0212	23	0.	0.	0.	0	*	1	1212	123	0.	6.	6.	6	*	1	2212	223	0.	1.1	1.1	
1	0218	24	0.	0.	0.	0	*	1	1218	124	0.	6.	6.	6	*	1	2218	224	0.	1.1	1.1	
1	0224	25	0.	0.	0.	0	*	1	1224	125	0.	6.	6.	6	*	1	2224	225	0.	1.1	1.1	
1	0230	26	0.	0.	0.	0	*	1	1230	126	0.	6.	6.	6	*	1	2230	226	0.	1.1	1.1	
1	0236	27	0.	0.	0.	0	*	1	1236	127	0.	6.	6.	6	*	1	2236	227	0.	1.1	1.1	
1	0242	28	0.	0.	0.	0	*	1	1242	128	0.	6.	6.	6	*	1	2242	228	0.	1.1	1.1	
1	0248	29	0.	0.	0.	0	*	1	1248	129	0.	7.	7.	7	*	1	2248	229	0.	1.1	1.1	
1	0254	30	0.	0.	0.	0	*	1	1254	130	0.	7.	7.	7	*	1	2254	230	0.	1.1	1.1	
1	0300	31	0.	0.	0.	0	*	1	1300	131	0.	7.	7.	7	*	1	2300	231	0.	1.1	1.1	
1	0306	32	0.	0.	0.	0	*	1	1306	132	0.	7.	7.	7	*	1	2306	232	0.	1.1	1.1	
1	0312	33	0.	0.	0.	0	*	1	1312	133	0.	7.	7.	7	*	1	2312	233	0.	1.1	1.1	
1	0318	34	0.	0.	0.	0	*	1	1318	134	0.	7.	7.	7	*	1	2318	234	0.	1.1	1.1	
1	0324	35	0.	0.	0.	0	*	1	1324	135	0.	7.	7.	7	*	1	2324	235	0.	1.1	1.1	
1	0330	36	0.	0.	0.	0	*	1	1330	136	0.	7.	7.	7	*	1	2330	236	0.	1.1	1.1	
1	0336	37	0.	0.	0.	0	*	1	1336	137	0.	7.	7.	7	*	1	2336	237	0.	1.1	1.1	
1	0342	38	0.	0.	0.	0	*	1	1342	138	0.	8.	8.	8	*	1	2342	238	0.	1.2	1.2	
1	0348	39	0.	0.	0.	0	*	1	1348	139	0.	8.	8.	8	*	1	2348	239	0.	1.2	1.2	
1	0354	40	0.	0.	0.	0	*	1	1354	140	0.	8.	8.	8	*	1	2354	240	0.	1.2	1.2	
1	0400	41	0.	0.	0.	0	*	1	1400	141	0.	8.	8.	8	*	2	0000	241	0.	1.2	1.2	
1	0406	42	0.	0.	0.	0	*	1	1406	142	0.	8.	8.	8	*	2	0006	242	0.	1.2	1.2	
1	0412	43	0.	0.	0.	0	*	1	1412	143	0.	8.	8.	8	*	2	0012	243	0.	1.2	1.2	
1	0418	44	0.	0.	0.	0	*	1	1418	144	0.	8.	8.	8	*	2	0018	244	0.	1.2	1.2	
1	0424	45	0.	0.	0.	0	*	1	1424	145	0.	8.	8.	8	*	2	0024	245	0.	1.2	1.2	
1	0430	46	0.	0.	0.	0	*	1	1430	146	0.	8.	8.	8	*	2	0030	246	0.	1.2	1.2	
1	0436	47	0.	0.	0.	0	*	1	1436	147	0.	8.	8.	8	*	2	0036	247	0.	1.2	1.2	
1	0442	48	0.	0.	0.	0	*	1	1442	148	0.	8.	8.	8	*	2	0042	248	0.	1.2	1.2	
1	0448	49	0.	0.	0.	0	*	1	1448	149	0.	8.	8.	8	*	2	0048	249	0.	1.2	1.2	
1	0454	50	0.	0.	0.	0	*	1	1454	150	0.	8.	8.	8	*	2	0054	250	0.	1.2	1.2	
1	0500	51	0.	0.	0.	0	*	1	1500	151	0.	8.	8.	8	*	2	0100	251	0.	1.2	1.2	
1	0506	52	0.	0.	0.	0	*	1	1506	152	0.	8.	8.	8	*	2	0106	252	0.	1.2	1.2	
1	0512	53	0.	0.	0.	0	*	1	1512	153	0.	9.	9.	9	*	2	0112	253	0.	1.2	1.2	
1	0518	54	0.	0.	0.	0	*	1	1518	154	0.	9.	9.	9	*	2	0118	254	0.	1.2	1.2	
1	0524	55	0.	0.	0.	0	*	1	1524	155	0.	9.	9.	9	*	2	0124	255	0.	1.2	1.2	
1	0530	56	0.	0.	0.	0	*	1	1530	156	0.	9.	9.	9	*	2	0130	256	0.	1.2	1.2	
1	0536	57	0.	0.	0.	0	*	1	1536	157	0.	9.	9.	9	*	2	0136	257	0.	1.2	1.2	
1	0542	58	0.	0.	0.	0	*	1	1542	158	0.	9.	9.	9	*	2	0142	258	0.	1.2	1.2	
1	0548	59	0.	0.	0.	0	*	1	1548	159	0.	9.	9.	9	*	2	0148	259	0.	1.2	1.2	
1	0554	60	0.	0.	0.	0	*	1	1554	160	0.	9.	9.	9	*	2	0154	260	0.	1.2	1.2	
1	0600	61	0.	0.	0.	0	*	1	1600	161	0.	9.	9.	9	*	2	0200	261	0.	1.2	1.2	
1	0606	62	0.	0.	0.	0	*	1	1606	162	0.	9.	9.	9	*	2	0206	262	0.	1.2	1.2	
1	0612	63	0.	0.	0.	0	*	1	1612	163	0.	9.	9.	9	*	2	0212	263	0.	1.2	1.2	
1	0618	64	0.	0.	0.	0	*	1	1618	164	0.	9.	9.	9	*	2	0218	264	0.	1.2	1.2	
1	0624	65	0.	0.	0.	0	*	1	1624	165	0.	9.	9.	9	*	2	0224	265	0.	1.2	1.2	
1	0630	66	0.	0.	0.	0	*	1	1630	166	0.	9.	9.	9	*	2	0230	266	0.	1.2	1.2	
1	0636	67	0.	0.	0.	0	*	1	1636	167	0.	9.	9.	9	*	2	0236	267	0.	1.2	1.2	
1	0642	68	0.	0.	0.	0	*	1	1642	168	0.	9.	9.	9	*	2	0242	268	0.	1.2	1.	

1	0748	79	0.	.0	.0 * 1	1748	179	0.	1.0	1.0 * 2	0348	279	0.	1.2	1.2
1	0754	80	0.	.0	.0 * 1	1754	180	0.	1.0	1.0 * 2	0354	280	0.	1.2	1.2
1	0800	81	0.	.0	.0 * 1	1800	181	0.	1.0	1.0 * 2	0400	281	0.	1.2	1.2
1	0806	82	0.	.0	.0 * 1	1806	182	0.	1.0	1.0 * 2	0406	282	0.	1.2	1.2
1	0812	83	0.	.0	.0 * 1	1812	183	0.	1.0	1.0 * 2	0412	283	0.	1.2	1.2
1	0818	84	0.	.0	.0 * 1	1818	184	0.	1.0	1.0 * 2	0418	284	0.	1.2	1.2
1	0824	85	0.	.0	.0 * 1	1824	185	0.	1.0	1.0 * 2	0424	285	0.	1.2	1.2
1	0830	86	0.	.0	.0 * 1	1830	186	0.	1.0	1.0 * 2	0430	286	0.	1.2	1.2
1	0836	87	0.	.0	.0 * 1	1836	187	0.	1.0	1.0 * 2	0436	287	0.	1.2	1.2
1	0842	88	0.	.0	.0 * 1	1842	188	0.	1.0	1.0 * 2	0442	288	0.	1.2	1.2
1	0848	89	0.	.0	.0 * 1	1848	189	0.	1.0	1.0 * 2	0448	289	0.	1.2	1.2
1	0854	90	0.	.0	.0 * 1	1854	190	0.	1.0	1.0 * 2	0454	290	0.	1.2	1.2
1	0900	91	0.	.0	.0 * 1	1900	191	0.	1.0	1.0 * 2	0500	291	0.	1.2	1.2
1	0906	92	0.	.0	.0 * 1	1906	192	0.	1.0	1.0 * 2	0506	292	0.	1.2	1.2
1	0912	93	0.	.0	.0 * 1	1912	193	0.	1.0	1.0 * 2	0512	293	0.	1.2	1.2
1	0918	94	0.	.0	.0 * 1	1918	194	0.	1.0	1.0 * 2	0518	294	0.	1.2	1.2
1	0924	95	0.	.0	.0 * 1	1924	195	0.	1.0	1.0 * 2	0524	295	0.	1.2	1.2
1	0930	96	0.	.0	.0 * 1	1930	196	0.	1.0	1.0 * 2	0530	296	0.	1.2	1.2
1	0936	97	0.	.0	.0 * 1	1936	197	0.	1.0	1.0 * 2	0536	297	0.	1.2	1.2
1	0942	98	0.	.0	.0 * 1	1942	198	0.	1.0	1.0 * 2	0542	298	0.	1.2	1.2
1	0948	99	0.	.0	.0 * 1	1948	199	0.	1.0	1.0 * 2	0548	299	0.	1.2	1.2
1	0954	100	0.	.0	.0 * 1	1954	200	0.	1.1	1.1 * 2	0554	300	0.	1.2	1.2

PEAK FLOW	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(HR)				
+	0.	.00				
		(CFS)				
		(INCHES)	.000	.000	.000	.000
		(AC-FT)	0.	0.	0.	0.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(AC-FT)	(HR)				
+	1.	23.90	1.	1.	1.	1.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(FEET)	(HR)				
+	1.15	24.40	1.15	.76	.61	.61

CUMULATIVE AREA = .03 SQ MI

206 K0 OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

205 HC HYDROGRAPH COMBINATION

ICOMP	3	NUMBER OF HYDROGRAPHS TO COMBINE
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HYDROGRAPH AT STATION SWB8
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(HR)				
+	10.	11.70				
		(CFS)				
		(INCHES)	.590	.724	.724	.724
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = .02 SQ MI

209 K0 OUTPUT CONTROL VARIABLES

IPRNT	1	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

210 RS STORAGE ROUTING

NSTPS	1	NUMBER OF SUBREACHES
ITYP	STOR	TYPE OF INITIAL CONDITION
RSVRIC	-1.00	INITIAL CONDITION
X	.00	WORKING R AND D COEFFICIENT

211 SV STORAGE .0 5000.0

212 SE ELEVATION .00 5000.00

213 SQ DISCHARGE 0. 1.

HYDROGRAPH AT STATION RSWB8 PLAN 1, RATIO = 1.00																						

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1		0000	1	0.	.0	.0	*	1		1000	101	0.	.0	.0	*	1		2000	201	0.	.6	.6
1		0006	2	0.	.0	.0	*	1		1006	102	0.	.0	.0	*	1		2006	202	0.	.6	.6
1		0012	3	0.	.0	.0	*	1		1012	103	0.	.0	.0	*	1		2012	203	0.	.6	.6
1		0018	4	0.	.0	.0	*	1		1018	104	0.	.0	.0	*	1		2018	204	0.	.6	.6
1		0024	5	0.	.0	.0	*	1		1024	105	0.	.0	.0	*	1		2024	205	0.	.6	.6
1		0030	6	0.	.0	.0	*	1		1030	106	0.	.0	.0	*	1		2030	206	0.	.6	.6
1		0036	7	0.	.0	.0	*	1		1036	107	0.	.0	.0	*	1		2036	207	0.	.6	.6
1		0042	8	0.	.0	.0	*	1		1042	108	0.	.0	.0	*	1		2042	208	0.	.6	.6
1		0048	9	0.	.0	.0	*	1		1048	109	0.	.0	.0	*	1		2048	209	0.	.6	.6
1		0054	10	0.	.0	.0	*	1		1054	110	0.	.0	.0	*	1		2054	210	0.	.6	.6
1		0100	11	0.	.0	.0	*	1		1100	111	0.	.0	.0	*	1		2100	211	0.	.6	.6
1		0106	12	0.	.0	.0	*	1		1106	112	0.	.0	.0	*	1		2106	212	0.	.6	.6
1		0112	13	0.	.0	.0	*	1		1112	113	0.	.0	.0	*	1		2112	213	0.	.6	.6
1		0118	14	0.	.0	.0	*	1		1118	114	0.	.0	.0	*	1		2118	214	0.	.6	.6
1		0124	15	0.	.0	.0	*	1		1124	115	0.	.0	.0	*	1		2124	215	0.	.6	.6
1		0130	16	0.	.0	.0	*	1		1130	116	0.	.0	.0	*	1		2130	216	0.	.6	.6
1		0136	17	0.	.0	.0	*	1		1136	117	0.	.1	.1	*	1		2136	217	0.	.6	.6
1		0142	18	0.	.0	.0	*	1		1142	118	0.	.2	.2	*	1		2142	218	0.	.6	.6
1		0148	19	0.	.0	.0	*	1		1148	119	0.	.2	.2	*	1		2148	219	0.	.6	.6
1		0154	20	0.	.0	.0	*	1		1154	120	0.	.3	.3	*	1		2154	220	0.	.6	.6
1		0200	21	0.	.0	.0	*	1		1200	121	0.	.3	.3	*	1		2200	221	0.	.6	.6
1		0206	22	0.	.0	.0	*	1		1206	122	0.	.3	.3	*	1		2206	222	0.	.6	.6
1		0212	23	0.	.0	.0	*	1		1212	123	0.	.3	.3	*	1		2212	223	0.	.6	.6
1		0218	24	0.	.0	.0	*	1		1218	124	0.	.3	.3	*	1		2218	224	0.	.6	.6
1		0224	25	0.	.0	.0	*	1		1224	125	0.	.3	.3	*	1		2224	225	0.	.6	.6
1		0230	26	0.	.0	.0	*	1		1230	126	0.	.3	.3	*	1		2230	226	0.	.6	.6
1		0236	27	0.	.0	.0	*	1		1236	127	0.	.3	.3	*	1		2236	227	0.	.6	.6
1		0242	28	0.	.0	.0	*	1		1242	128	0.	.4	.4	*	1		2242	228	0.	.6	.6
1		0248	29	0.	.0	.0	*	1		1248	129	0.	.4	.4	*	1		2248	229	0.	.6	.6
1		0254	30	0.	.0	.0	*	1		1254	130	0.	.4	.4	*	1		2254	230	0.	.6	.6
1		0300	31	0.	.0	.0	*	1		1300	131	0.	.4	.4	*	1		2300	231	0.	.6	.6
1		0306	32	0.	.0	.0	*	1		1306	132	0.	.4	.4	*	1		2306	232	0.	.6	.6
1		0312	33	0.	.0	.0	*	1		1312	133	0.	.4	.4	*	1		2312	233	0.	.6	.6
1		0318	34	0.	.0	.0	*	1		1318	134	0.	.4	.4	*	1		2318	234	0.	.6	.6
1		0324	35	0.	.0	.0	*	1		1324	135	0.	.4	.4	*	1		2324	235	0.	.6	.6
1		0330	36	0.	.0	.0	*	1		1330	136	0.	.4	.4	*	1		2330	236	0.	.6	.6
1		0336	37	0.	.0	.0	*	1		1336	137	0.	.4	.4	*	1		2336	237	0.	.6	.6
1		0342	38	0.	.0	.0	*	1		1342	138	0.	.4	.4	*	1		2342	238	0.	.6	.6
1		0348	39	0.	.0	.0	*	1		1348	139	0.	.4	.4	*	1		2348	239	0.	.6	.6
1		0354	40	0.	.0	.0	*	1		1354	140	0.	.4	.4	*	1		2354	240	0.	.6	.6
1		0400	41	0.	.0	.0	*	1		1400	141	0.	.4	.4	*	2		0000	241	0.	.6	.6
1		0406	42	0.	.0	.0	*	1		1406	142	0.	.4	.4	*	2		0006	242	0.	.6	.6
1		0412	43	0.	.0	.0	*	1		1412	143	0.	.4	.4	*	2		0012	243	0.	.6	.6
1		0418	44	0.	.0	.0	*	1		1418	144	0.	.4	.4	*	2		0018	244	0.	.6	.6
1		0424	45	0.	.0	.0	*	1		1424	145	0.	.4	.4	*	2		0024	245	0.	.6	.6
1		0430	46	0.	.0	.0	*	1		1430	146	0.	.4	.4	*	2		0030	246	0.	.6	.6
1		0436	47	0.	.0	.0	*	1		1436	147	0.	.4	.4	*	2		0036	247	0.	.6	.6
1		0442	48	0.	.0	.0	*	1		1442	148	0.	.4	.4	*	2		0042	248	0.	.6	.6
1		0448	49	0.	.0	.0	*	1		1448	149	0.	.4	.4	*	2		0048	249	0.	.6	.6
1		0454	50	0.	.0	.0	*	1		1454	150	0.	.5	.5	*	2		0054	250	0.	.6	.6
1		0500	51	0.	.0	.0	*	1		1500	151	0.	.5	.5	*	2		0100	251	0.	.6	.6
1		0506	52	0.	.0	.0	*	1		1506	152	0.	.5	.5	*	2		0106	252	0.	.6	.6
1		0512	53	0.	.0	.0	*	1		1512	153	0.	.5	.5	*	2		0112	253	0.	.6	.6
1		0518	54	0.	.0	.0	*	1		1518	154	0.	.5	.5	*	2		0118	254	0.	.6	.6
1		0524	55	0.	.0	.0	*	1		1524	155	0.	.5	.5	*	2		0124	255	0.	.6	.6
1		0530	56	0.	.0	.0	*	1		1530	156	0.	.5	.5	*	2		0130	256	0.	.6	.6
1		0536	57	0.	.0	.0	*	1		1536	157	0.	.5	.5	*	2		0136	257	0.	.6	.6
1		0542	58	0.	.0	.0	*	1		1542	158	0.	.5	.5	*	2		0142	258	0.	.6	.6
1		0548	59	0.	.0	.0	*	1		1548	159	0.	.5	.5	*	2		0148	259	0.	.6	.6
1		0554	60	0.	.0	.0	*	1		1554	160	0.	.5	.5	*	2		0154	260	0.	.6	.6
1		0600	61	0.	.0	.0	*	1		1600	161	0.	.5	.5	*	2		0200	261	0.	.6	.6
1		0606	62	0.	.0	.0	*	1		1606	162	0.	.5	.5	*	2		0206	262	0.	.6	.6
1		0612	63	0.	.0	.0	*	1		1612	163	0.	.5	.5	*	2		0212	263	0.	.6	.6
1		0618	64	0.	.0	.0	*	1		1618	164	0.	.5	.5	*	2		0218	264	0.	.6	.6
1		0624	65	0.	.0	.0	*	1		1624	165	0.	.5	.5	*	2		0224	265	0.	.6	.6
1		0630	66	0.	.0	.0	*	1		1630	166	0.	.5	.5	*	2		0230	266	0.	.6	.6
1		0636	67	0.	.0	.0	*	1		1636	167	0.	.5	.5	*	2		0236	267	0.	.6	.6
1		0642	68	0.	.0	.0	*	1		1642	168	0.	.5	.5	*	2		0242	268	0.	.6	.6
1		0648	69	0.	.0	.0	*	1		1648	169	0.	.5	.5	*	2		0248	269	0.	.6	.6
1		0654	70	0.	.0	.0	*	1		1654	170	0.	.5	.5	*	2		0254	270	0.	.6	.6
1		0700	71	0.	.0	.0	*	1		1700	171	0.	.5	.5	*	2		0300	271	0.	.6	.6
1		0706	72	0.	.0	.0	*	1		1706	172	0.	.5	.5	*	2		0306	272	0.	.6	.6
1		0712	73	0.	.0	.0	*	1		1712	173											

1	0824	85	0.	.0	.0 *	1	1824	185	0.	.5	.5 *	2	0424	285	0.	.6	.6
1	0830	86	0.	.0	.0 *	1	1830	186	0.	.5	.5 *	2	0430	286	0.	.6	.6
1	0836	87	0.	.0	.0 *	1	1836	187	0.	.5	.5 *	2	0436	287	0.	.6	.6
1	0842	88	0.	.0	.0 *	1	1842	188	0.	.5	.5 *	2	0442	288	0.	.6	.6
1	0848	89	0.	.0	.0 *	1	1848	189	0.	.5	.5 *	2	0448	289	0.	.6	.6
1	0854	90	0.	.0	.0 *	1	1854	190	0.	.5	.5 *	2	0454	290	0.	.6	.6
1	0900	91	0.	.0	.0 *	1	1900	191	0.	.6	.6 *	2	0500	291	0.	.6	.6
1	0906	92	0.	.0	.0 *	1	1906	192	0.	.6	.6 *	2	0506	292	0.	.6	.6
1	0912	93	0.	.0	.0 *	1	1912	193	0.	.6	.6 *	2	0512	293	0.	.6	.6
1	0918	94	0.	.0	.0 *	1	1918	194	0.	.6	.6 *	2	0518	294	0.	.6	.6
1	0924	95	0.	.0	.0 *	1	1924	195	0.	.6	.6 *	2	0524	295	0.	.6	.6
1	0930	96	0.	.0	.0 *	1	1930	196	0.	.6	.6 *	2	0530	296	0.	.6	.6
1	0936	97	0.	.0	.0 *	1	1936	197	0.	.6	.6 *	2	0536	297	0.	.6	.6
1	0942	98	0.	.0	.0 *	1	1942	198	0.	.6	.6 *	2	0542	298	0.	.6	.6
1	0948	99	0.	.0	.0 *	1	1948	199	0.	.6	.6 *	2	0548	299	0.	.6	.6
1	0954	100	0.	.0	.0 *	1	1954	200	0.	.6	.6 *	2	0554	300	0.	.6	.6

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+	(CFS)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	0.	.00	0.	0.	0.	0.
		(INCHES)	.000	.000	.000	.000
		(AC-FT)	0.	0.	0.	0.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	1.	23.70	1.	0.	0.	0.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+	(FEET)	(HR)	6-HR	24-HR	72-HR	29.90-HR
+	.62	24.00	.62	.41	.33	.33

CUMULATIVE AREA = .02 SQ MI

221 K0 OUTPUT CONTROL VARIABLES
 IPRNT 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

222 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC -1.00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

223 SV STORAGE .0 5000.0

224 SE ELEVATION .00 5000.00

225 SQ DISCHARGE 0. 1.

HYDROGRAPH AT STATION RSUB12
PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1	0000	1	0.	.0	.0 *	1		1000	101	0.	.0	.0 *	1	2000	201	0.	.1	.1				
1	0006	2	0.	.0	.0 *	1		1006	102	0.	.0	.0 *	1	2006	202	0.	.1	.1				
1	0012	3	0.	.0	.0 *	1		1012	103	0.	.0	.0 *	1	2012	203	0.	.1	.1				
1	0018	4	0.	.0	.0 *	1		1018	104	0.	.0	.0 *	1	2018	204	0.	.1	.1				
1	0024	5	0.	.0	.0 *	1		1024	105	0.	.0	.0 *	1	2024	205	0.	.1	.1				
1	0030	6	0.	.0	.0 *	1		1030	106	0.	.0	.0 *	1	2030	206	0.	.1	.1				
1	0036	7	0.	.0	.0 *	1		1036	107	0.	.0	.0 *	1	2036	207	0.	.1	.1				
1	0042	8	0.	.0	.0 *	1		1042	108	0.	.0	.0 *	1	2042	208	0.	.1	.1				
1	0048	9	0.	.0	.0 *	1		1048	109	0.	.0	.0 *	1	2048	209	0.	.1	.1				
1	0054	10	0.	.0	.0 *	1		1054	110	0.	.0	.0 *	1	2054	210	0.	.1	.1				
1	0100	11	0.	.0	.0 *	1		1100	111	0.	.0	.0 *	1	2100	211	0.	.1	.1				
1	0106	12	0.	.0	.0 *	1		1106	112	0.	.0	.0 *	1	2106	212	0.	.1	.1				
1	0112	13	0.	.0	.0 *	1		1112	113	0.	.0	.0 *	1	2112	213	0.	.1	.1				
1	0118	14	0.	.0	.0 *	1		1118	114	0.	.0	.0 *	1	2118	214	0.	.1	.1				
1	0124	15	0.	.0	.0 *	1		1124	115	0.	.0	.0 *	1	2124	215	0.	.1	.1				
1	0130	16	0.	.0	.0 *	1		1130	116	0.	.0	.0 *	1	2130	216	0.	.1	.1				
1	0136	17	0.	.0	.0 *	1		1136	117	0.	.0	.0 *	1	2136	217	0.	.1	.1				
1	0142	18	0.	.0	.0 *	1		1142	118	0.	.0	.0 *	1	2142	218	0.	.1	.1				
1	0148	19	0.	.0	.0 *	1		1148	119	0.	.0	.0 *	1	2148	219	0.	.1	.1				
1	0154	20	0.	.0	.0 *	1		1154	120	0.	.0	.0 *	1	2154	220	0.	.1	.1				
1	0200	21	0.	.0	.0 *	1		1200	121	0.	.1	.1 *	1	2200	221	0.	.1	.1				
1	0206	22	0.	.0	.0 *	1		1206	122	0.	.1	.1 *	1	2206	222	0.	.1	.1				
1	0212	23	0.	.0	.0 *	1		1212	123	0.	.1	.1 *	1	2212	223	0.	.1	.1				

1	0218	24	0.	.0	.0	*	1	1218	124	0.	.1	.1	*	1	2218	224	0.	.1	.1
1	0224	25	0.	.0	.0	*	1	1224	125	0.	.1	.1	*	1	2224	225	0.	.1	.1
1	0230	26	0.	.0	.0	*	1	1230	126	0.	.1	.1	*	1	2230	226	0.	.1	.1
1	0236	27	0.	.0	.0	*	1	1236	127	0.	.1	.1	*	1	2236	227	0.	.1	.1
1	0242	28	0.	.0	.0	*	1	1242	128	0.	.1	.1	*	1	2242	228	0.	.1	.1
1	0248	29	0.	.0	.0	*	1	1248	129	0.	.1	.1	*	1	2248	229	0.	.1	.1
1	0254	30	0.	.0	.0	*	1	1254	130	0.	.1	.1	*	1	2254	230	0.	.1	.1
1	0300	31	0.	.0	.0	*	1	1300	131	0.	.1	.1	*	1	2300	231	0.	.1	.1
1	0306	32	0.	.0	.0	*	1	1306	132	0.	.1	.1	*	1	2306	232	0.	.1	.1
1	0312	33	0.	.0	.0	*	1	1312	133	0.	.1	.1	*	1	2312	233	0.	.1	.1
1	0318	34	0.	.0	.0	*	1	1318	134	0.	.1	.1	*	1	2318	234	0.	.1	.1
1	0324	35	0.	.0	.0	*	1	1324	135	0.	.1	.1	*	1	2324	235	0.	.1	.1
1	0330	36	0.	.0	.0	*	1	1330	136	0.	.1	.1	*	1	2330	236	0.	.1	.1
1	0336	37	0.	.0	.0	*	1	1336	137	0.	.1	.1	*	1	2336	237	0.	.1	.1
1	0342	38	0.	.0	.0	*	1	1342	138	0.	.1	.1	*	1	2342	238	0.	.1	.1
1	0348	39	0.	.0	.0	*	1	1348	139	0.	.1	.1	*	1	2348	239	0.	.1	.1
1	0354	40	0.	.0	.0	*	1	1354	140	0.	.1	.1	*	1	2354	240	0.	.1	.1
1	0400	41	0.	.0	.0	*	1	1400	141	0.	.1	.1	*	2	0000	241	0.	.1	.1
1	0406	42	0.	.0	.0	*	1	1406	142	0.	.1	.1	*	2	0006	242	0.	.1	.1
1	0412	43	0.	.0	.0	*	1	1412	143	0.	.1	.1	*	2	0012	243	0.	.1	.1
1	0418	44	0.	.0	.0	*	1	1418	144	0.	.1	.1	*	2	0018	244	0.	.1	.1
1	0424	45	0.	.0	.0	*	1	1424	145	0.	.1	.1	*	2	0024	245	0.	.1	.1
1	0430	46	0.	.0	.0	*	1	1430	146	0.	.1	.1	*	2	0030	246	0.	.1	.1
1	0436	47	0.	.0	.0	*	1	1436	147	0.	.1	.1	*	2	0036	247	0.	.1	.1
1	0442	48	0.	.0	.0	*	1	1442	148	0.	.1	.1	*	2	0042	248	0.	.1	.1
1	0448	49	0.	.0	.0	*	1	1448	149	0.	.1	.1	*	2	0048	249	0.	.1	.1
1	0454	50	0.	.0	.0	*	1	1454	150	0.	.1	.1	*	2	0054	250	0.	.1	.1
1	0500	51	0.	.0	.0	*	1	1500	151	0.	.1	.1	*	2	0100	251	0.	.1	.1
1	0506	52	0.	.0	.0	*	1	1506	152	0.	.1	.1	*	2	0106	252	0.	.1	.1
1	0512	53	0.	.0	.0	*	1	1512	153	0.	.1	.1	*	2	0112	253	0.	.1	.1
1	0518	54	0.	.0	.0	*	1	1518	154	0.	.1	.1	*	2	0118	254	0.	.1	.1
1	0524	55	0.	.0	.0	*	1	1524	155	0.	.1	.1	*	2	0124	255	0.	.1	.1
1	0530	56	0.	.0	.0	*	1	1530	156	0.	.1	.1	*	2	0130	256	0.	.1	.1
1	0536	57	0.	.0	.0	*	1	1536	157	0.	.1	.1	*	2	0136	257	0.	.1	.1
1	0542	58	0.	.0	.0	*	1	1542	158	0.	.1	.1	*	2	0142	258	0.	.1	.1
1	0548	59	0.	.0	.0	*	1	1548	159	0.	.1	.1	*	2	0148	259	0.	.1	.1
1	0554	60	0.	.0	.0	*	1	1554	160	0.	.1	.1	*	2	0154	260	0.	.1	.1
1	0600	61	0.	.0	.0	*	1	1600	161	0.	.1	.1	*	2	0200	261	0.	.1	.1
1	0606	62	0.	.0	.0	*	1	1606	162	0.	.1	.1	*	2	0206	262	0.	.1	.1
1	0612	63	0.	.0	.0	*	1	1612	163	0.	.1	.1	*	2	0212	263	0.	.1	.1
1	0618	64	0.	.0	.0	*	1	1618	164	0.	.1	.1	*	2	0218	264	0.	.1	.1
1	0624	65	0.	.0	.0	*	1	1624	165	0.	.1	.1	*	2	0224	265	0.	.1	.1
1	0630	66	0.	.0	.0	*	1	1630	166	0.	.1	.1	*	2	0230	266	0.	.1	.1
1	0636	67	0.	.0	.0	*	1	1636	167	0.	.1	.1	*	2	0236	267	0.	.1	.1
1	0642	68	0.	.0	.0	*	1	1642	168	0.	.1	.1	*	2	0242	268	0.	.1	.1
1	0648	69	0.	.0	.0	*	1	1648	169	0.	.1	.1	*	2	0248	269	0.	.1	.1
1	0654	70	0.	.0	.0	*	1	1654	170	0.	.1	.1	*	2	0254	270	0.	.1	.1
1	0700	71	0.	.0	.0	*	1	1700	171	0.	.1	.1	*	2	0300	271	0.	.1	.1
1	0706	72	0.	.0	.0	*	1	1706	172	0.	.1	.1	*	2	0306	272	0.	.1	.1
1	0712	73	0.	.0	.0	*	1	1712	173	0.	.1	.1	*	2	0312	273	0.	.1	.1
1	0718	74	0.	.0	.0	*	1	1718	174	0.	.1	.1	*	2	0318	274	0.	.1	.1
1	0724	75	0.	.0	.0	*	1	1724	175	0.	.1	.1	*	2	0324	275	0.	.1	.1
1	0730	76	0.	.0	.0	*	1	1730	176	0.	.1	.1	*	2	0330	276	0.	.1	.1
1	0736	77	0.	.0	.0	*	1	1736	177	0.	.1	.1	*	2	0336	277	0.	.1	.1
1	0742	78	0.	.0	.0	*	1	1742	178	0.	.1	.1	*	2	0342	278	0.	.1	.1
1	0748	79	0.	.0	.0	*	1	1748	179	0.	.1	.1	*	2	0348	279	0.	.1	.1
1	0754	80	0.	.0	.0	*	1	1754	180	0.	.1	.1	*	2	0354	280	0.	.1	.1
1	0800	81	0.	.0	.0	*	1	1800	181	0.	.1	.1	*	2	0400	281	0.	.1	.1
1	0806	82	0.	.0	.0	*	1	1806	182	0.	.1	.1	*	2	0406	282	0.	.1	.1
1	0812	83	0.	.0	.0	*	1	1812	183	0.	.1	.1	*	2	0412	283	0.	.1	.1
1	0818	84	0.	.0	.0	*	1	1818	184	0.	.1	.1	*	2	0418	284	0.	.1	.1
1	0824	85	0.	.0	.0	*	1	1824	185	0.	.1	.1	*	2	0424	285	0.	.1	.1
1	0830	86	0.	.0	.0	*	1	1830	186	0.	.1	.1	*	2	0430	286	0.	.1	.1
1	0836	87	0.	.0	.0	*	1	1836	187	0.	.1	.1	*	2	0436	287	0.	.1	.1
1	0842	88	0.	.0	.0	*	1	1842	188	0.	.1	.1	*	2	0442	288	0.	.1	.1
1	0848	89	0.	.0	.0	*	1	1848	189	0.	.1	.1	*	2	0448	289	0.	.1	.1
1	0854	90	0.	.0	.0	*	1	1854	190	0.	.1	.1	*	2	0454	290	0.	.1	.1
1	0900	91	0.	.0	.0	*	1	1900	191	0.	.1	.1	*	2	0500	291	0.	.1	.1
1	0906	92	0.	.0	.0	*	1	1906	192	0.	.1	.1	*	2	0506	292	0.	.1	.1
1	0912	93	0.	.0	.0	*	1	1912	193	0.	.1	.1	*	2	0512	293	0.	.1	.1
1	0918	94	0.	.0	.0	*	1	1918	194	0.	.1	.1	*	2	0518	294	0.	.1	.1
1	0924	95	0.	.0	.0	*	1	1924	195	0.	.1	.1	*	2	0524	295	0.	.1	.1
1	0930	96	0.	.0	.0	*	1	1930	196	0.	.1	.1	*	2	0530	296	0.	.1	.1
1	0936	97	0.	.0	.0	*	1	1936	197	0.	.1	.1	*	2	0536	297	0.	.1	.1
1	0942	98	0.	.0	.0	*	1	1942	198	0.	.1	.1	*	2	0542	298	0.	.1	.1
1	0948	99	0.	.0	.0	*	1	1948	199	0.	.1	.1	*	2	0548	299	0.	.1	.1
1	0954	100	0.	.0	.0	*	1	1954	200	0.	.1	.1	*	2	0554	300	0.	.1	.1

PEAK FLOW	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(HR)				
		(CFS)				
+	0.	.00	0.	0.	0.	0.
		(INCHES)	.000	.000	.000	.000
		(AC-FT)	0.	0.	0.	0.
PEAK STORAGE	TIME		6-HR	24-HR	72-HR	29.90-HR
+	(AC-FT)	(HR)				
	0.	23.50	0.	0.	0.	0.

CUMULATIVE AREA = .00 SQ MI

HYDROGRAPH ROUTING DATA

237 S0 DISCHARGE 0. 1.

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1		0000	1	0.	.0	.0	*	1		1000	101	0.	.0	.0	*	1		2000	201	0.	.2	.2
1		0006	2	0.	.0	.0	*	1		1006	102	0.	.0	.0	*	1		2006	202	0.	.2	.2
1		0012	3	0.	.0	.0	*	1		1012	103	0.	.0	.0	*	1		2012	203	0.	.2	.2
1		0018	4	0.	.0	.0	*	1		1018	104	0.	.0	.0	*	1		2018	204	0.	.2	.2
1		0024	5	0.	.0	.0	*	1		1024	105	0.	.0	.0	*	1		2024	205	0.	.2	.2
1		0030	6	0.	.0	.0	*	1		1030	106	0.	.0	.0	*	1		2030	206	0.	.2	.2
1		0036	7	0.	.0	.0	*	1		1036	107	0.	.0	.0	*	1		2036	207	0.	.2	.2
1		0042	8	0.	.0	.0	*	1		1042	108	0.	.0	.0	*	1		2042	208	0.	.2	.2
1		0048	9	0.	.0	.0	*	1		1048	109	0.	.0	.0	*	1		2048	209	0.	.2	.2
1		0054	10	0.	.0	.0	*	1		1054	110	0.	.0	.0	*	1		2054	210	0.	.2	.2
1		0100	11	0.	.0	.0	*	1		1100	111	0.	.0	.0	*	1		2100	211	0.	.2	.2
1		0106	12	0.	.0	.0	*	1		1106	112	0.	.0	.0	*	1		2106	212	0.	.2	.2
1		0112	13	0.	.0	.0	*	1		1112	113	0.	.0	.0	*	1		2112	213	0.	.2	.2
1		0118	14	0.	.0	.0	*	1		1118	114	0.	.0	.0	*	1		2118	214	0.	.2	.2
1		0124	15	0.	.0	.0	*	1		1124	115	0.	.0	.0	*	1		2124	215	0.	.2	.2
1		0130	16	0.	.0	.0	*	1		1130	116	0.	.0	.0	*	1		2130	216	0.	.2	.2
1		0136	17	0.	.0	.0	*	1		1136	117	0.	.0	.0	*	1		2136	217	0.	.2	.2
1		0142	18	0.	.0	.0	*	1		1142	118	0.	.1	.1	*	1		2142	218	0.	.2	.2
1		0148	19	0.	.0	.0	*	1		1148	119	0.	.1	.1	*	1		2148	219	0.	.2	.2
1		0154	20	0.	.0	.0	*	1		1154	120	0.	.1	.1	*	1		2154	220	0.	.2	.2
1		0200	21	0.	.0	.0	*	1		1200	121	0.	.1	.1	*	1		2200	221	0.	.2	.2
1		0206	22	0.	.0	.0	*	1		1206	122	0.	.1	.1	*	1		2206	222	0.	.2	.2
1		0212	23	0.	.0	.0	*	1		1212	123	0.	.1	.1	*	1		2212	223	0.	.2	.2
1		0218	24	0.	.0	.0	*	1		1218	124	0.	.1	.1	*	1		2218	224	0.	.2	.2
1		0224	25	0																		

1	0530	56	0.	.0	.0 *	1	1530	156	0.	.2	.2 *	2	0130	256	0.	.2	.2
1	0536	57	0.	.0	.0 *	1	1536	157	0.	.2	.2 *	2	0136	257	0.	.2	.2
1	0542	58	0.	.0	.0 *	1	1542	158	0.	.2	.2 *	2	0142	258	0.	.2	.2
1	0548	59	0.	.0	.0 *	1	1548	159	0.	.2	.2 *	2	0148	259	0.	.2	.2
1	0554	60	0.	.0	.0 *	1	1554	160	0.	.2	.2 *	2	0154	260	0.	.2	.2
1	0600	61	0.	.0	.0 *	1	1600	161	0.	.2	.2 *	2	0200	261	0.	.2	.2
1	0606	62	0.	.0	.0 *	1	1606	162	0.	.2	.2 *	2	0206	262	0.	.2	.2
1	0612	63	0.	.0	.0 *	1	1612	163	0.	.2	.2 *	2	0212	263	0.	.2	.2
1	0618	64	0.	.0	.0 *	1	1618	164	0.	.2	.2 *	2	0218	264	0.	.2	.2
1	0624	65	0.	.0	.0 *	1	1624	165	0.	.2	.2 *	2	0224	265	0.	.2	.2
1	0630	66	0.	.0	.0 *	1	1630	166	0.	.2	.2 *	2	0230	266	0.	.2	.2
1	0636	67	0.	.0	.0 *	1	1636	167	0.	.2	.2 *	2	0236	267	0.	.2	.2
1	0642	68	0.	.0	.0 *	1	1642	168	0.	.2	.2 *	2	0242	268	0.	.2	.2
1	0648	69	0.	.0	.0 *	1	1648	169	0.	.2	.2 *	2	0248	269	0.	.2	.2
1	0654	70	0.	.0	.0 *	1	1654	170	0.	.2	.2 *	2	0254	270	0.	.2	.2
1	0700	71	0.	.0	.0 *	1	1700	171	0.	.2	.2 *	2	0300	271	0.	.2	.2
1	0706	72	0.	.0	.0 *	1	1706	172	0.	.2	.2 *	2	0306	272	0.	.2	.2
1	0712	73	0.	.0	.0 *	1	1712	173	0.	.2	.2 *	2	0312	273	0.	.2	.2
1	0718	74	0.	.0	.0 *	1	1718	174	0.	.2	.2 *	2	0318	274	0.	.2	.2
1	0724	75	0.	.0	.0 *	1	1724	175	0.	.2	.2 *	2	0324	275	0.	.2	.2
1	0730	76	0.	.0	.0 *	1	1730	176	0.	.2	.2 *	2	0330	276	0.	.2	.2
1	0736	77	0.	.0	.0 *	1	1736	177	0.	.2	.2 *	2	0336	277	0.	.2	.2
1	0742	78	0.	.0	.0 *	1	1742	178	0.	.2	.2 *	2	0342	278	0.	.2	.2
1	0748	79	0.	.0	.0 *	1	1748	179	0.	.2	.2 *	2	0348	279	0.	.2	.2
1	0754	80	0.	.0	.0 *	1	1754	180	0.	.2	.2 *	2	0354	280	0.	.2	.2
1	0800	81	0.	.0	.0 *	1	1800	181	0.	.2	.2 *	2	0400	281	0.	.2	.2
1	0806	82	0.	.0	.0 *	1	1806	182	0.	.2	.2 *	2	0406	282	0.	.2	.2
1	0812	83	0.	.0	.0 *	1	1812	183	0.	.2	.2 *	2	0412	283	0.	.2	.2
1	0818	84	0.	.0	.0 *	1	1818	184	0.	.2	.2 *	2	0418	284	0.	.2	.2
1	0824	85	0.	.0	.0 *	1	1824	185	0.	.2	.2 *	2	0424	285	0.	.2	.2
1	0830	86	0.	.0	.0 *	1	1830	186	0.	.2	.2 *	2	0430	286	0.	.2	.2
1	0836	87	0.	.0	.0 *	1	1836	187	0.	.2	.2 *	2	0436	287	0.	.2	.2
1	0842	88	0.	.0	.0 *	1	1842	188	0.	.2	.2 *	2	0442	288	0.	.2	.2
1	0848	89	0.	.0	.0 *	1	1848	189	0.	.2	.2 *	2	0448	289	0.	.2	.2
1	0854	90	0.	.0	.0 *	1	1854	190	0.	.2	.2 *	2	0454	290	0.	.2	.2
1	0900	91	0.	.0	.0 *	1	1900	191	0.	.2	.2 *	2	0500	291	0.	.2	.2
1	0906	92	0.	.0	.0 *	1	1906	192	0.	.2	.2 *	2	0506	292	0.	.2	.2
1	0912	93	0.	.0	.0 *	1	1912	193	0.	.2	.2 *	2	0512	293	0.	.2	.2
1	0918	94	0.	.0	.0 *	1	1918	194	0.	.2	.2 *	2	0518	294	0.	.2	.2
1	0924	95	0.	.0	.0 *	1	1924	195	0.	.2	.2 *	2	0524	295	0.	.2	.2
1	0930	96	0.	.0	.0 *	1	1930	196	0.	.2	.2 *	2	0530	296	0.	.2	.2
1	0936	97	0.	.0	.0 *	1	1936	197	0.	.2	.2 *	2	0536	297	0.	.2	.2
1	0942	98	0.	.0	.0 *	1	1942	198	0.	.2	.2 *	2	0542	298	0.	.2	.2
1	0948	99	0.	.0	.0 *	1	1948	199	0.	.2	.2 *	2	0548	299	0.	.2	.2
1	0954	100	0.	.0	.0 *	1	1954	200	0.	.2	.2 *	2	0554	300	0.	.2	.2

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	29.90-HR
+	(CFS)	(HR)				
+	0.	.00	0.	0.	0.	0.
		(INCHES)	.000	.000	.000	.000
		(AC-FT)	0.	0.	0.	0.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
			6-HR	24-HR	72-HR	29.90-HR
+	(AC-FT)	(HR)				
	0.	23.60	0.	0.	0.	0.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
			6-HR	24-HR	72-HR	29.90-HR
+	(FEET)	(HR)				
	.20	24.00	.20	.13	.11	.11

CUMULATIVE AREA = .01 SQ MI

245 KO OUTPUT CONTROL VARIABLES

IPRNT	1	PRINT CONTROL
IPLST	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

246 RS STORAGE ROUTING

NSTPS	1	NUMBER OF SUBREACHES
ITYP	STOR	TYPE OF INITIAL CONDITION
RSVRIC	-1.00	INITIAL CONDITION
X	.00	WORKING R AND D COEFFICIENT

247 SV STORAGE .0 5000.0

248 SE ELEVATION .00 5000.00

249 SQ DISCHARGE 0. 1.

HYDROGRAPH AT STATION RSUB3A

PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1	0000	1	0.	.0	.0	.0	*	1	1000	101	0.	.0	.0	.0	*	1	2000	201	0.	.1	.1	.1
1	0006	2	0.	.0	.0	.0	*	1	1006	102	0.	.0	.0	.0	*	1	2006	202	0.	.1	.1	.1
1	0012	3	0.	.0	.0	.0	*	1	1012	103	0.	.0	.0	.0	*	1	2012	203	0.	.1	.1	.1
1	0018	4	0.	.0	.0	.0	*	1	1018	104	0.	.0	.0	.0	*	1	2018	204	0.	.1	.1	.1
1	0024	5	0.	.0	.0	.0	*	1	1024	105	0.	.0	.0	.0	*	1	2024	205	0.	.1	.1	.1
1	0030	6	0.	.0	.0	.0	*	1	1030	106	0.	.0	.0	.0	*	1	2030	206	0.	.2	.2	.2
1	0036	7	0.	.0	.0	.0	*	1	1036	107	0.	.0	.0	.0	*	1	2036	207	0.	.2	.2	.2
1	0042	8	0.	.0	.0	.0	*	1	1042	108	0.	.0	.0	.0	*	1	2042	208	0.	.2	.2	.2
1	0048	9	0.	.0	.0	.0	*	1	1048	109	0.	.0	.0	.0	*	1	2048	209	0.	.2	.2	.2
1	0054	10	0.	.0	.0	.0	*	1	1054	110	0.	.0	.0	.0	*	1	2054	210	0.	.2	.2	.2
1	0100	11	0.	.0	.0	.0	*	1	1100	111	0.	.0	.0	.0	*	1	2100	211	0.	.2	.2	.2
1	0106	12	0.	.0	.0	.0	*	1	1106	112	0.	.0	.0	.0	*	1	2106	212	0.	.2	.2	.2
1	0112	13	0.	.0	.0	.0	*	1	1112	113	0.	.0	.0	.0	*	1	2112	213	0.	.2	.2	.2
1	0118	14	0.	.0	.0	.0	*	1	1118	114	0.	.0	.0	.0	*	1	2118	214	0.	.2	.2	.2
1	0124	15	0.	.0	.0	.0	*	1	1124	115	0.	.0	.0	.0	*	1	2124	215	0.	.2	.2	.2
1	0130	16	0.	.0	.0	.0	*	1	1130	116	0.	.0	.0	.0	*	1	2130	216	0.	.2	.2	.2
1	0136	17	0.	.0	.0	.0	*	1	1136	117	0.	.0	.0	.0	*	1	2136	217	0.	.2	.2	.2
1	0142	18	0.	.0	.0	.0	*	1	1142	118	0.	.0	.0	.0	*	1	2142	218	0.	.2	.2	.2
1	0148	19	0.	.0	.0	.0	*	1	1148	119	0.	.0	.0	.0	*	1	2148	219	0.	.2	.2	.2
1	0154	20	0.	.0	.0	.0	*	1	1154	120	0.	.1	.1	.1	*	1	2154	220	0.	.2	.2	.2
1	0200	21	0.	.0	.0	.0	*	1	1200	121	0.	.1	.1	.1	*	1	2200	221	0.	.2	.2	.2
1	0206	22	0.	.0	.0	.0	*	1	1206	122	0.	.1	.1	.1	*	1	2206	222	0.	.2	.2	.2
1	0212	23	0.	.0	.0	.0	*	1	1212	123	0.	.1	.1	.1	*	1	2212	223	0.	.2	.2	.2
1	0218	24	0.	.0	.0	.0	*	1	1218	124	0.	.1	.1	.1	*	1	2218	224	0.	.2	.2	.2
1	0224	25	0.	.0	.0	.0	*	1	1224	125	0.	.1	.1	.1	*	1	2224	225	0.	.2	.2	.2
1	0230	26	0.	.0	.0	.0	*	1	1230	126	0.	.1	.1	.1	*	1	2230	226	0.	.2	.2	.2
1	0236	27	0.	.0	.0	.0	*	1	1236	127	0.	.1	.1	.1	*	1	2236	227	0.	.2	.2	.2
1	0242	28	0.	.0	.0	.0	*	1	1242	128	0.	.1	.1	.1	*	1	2242	228	0.	.2	.2	.2
1	0248	29	0.	.0	.0	.0	*	1	1248	129	0.	.1	.1	.1	*	1	2248	229	0.	.2	.2	.2
1	0254	30	0.	.0	.0	.0	*	1	1254	130	0.	.1	.1	.1	*	1	2254	230	0.	.2	.2	.2
1	0300	31	0.	.0	.0	.0	*	1	1300	131	0.	.1	.1	.1	*	1	2300	231	0.	.2	.2	.2
1	0306	32	0.	.0	.0	.0	*	1	1306	132	0.	.1	.1	.1	*	1	2306	232	0.	.2	.2	.2
1	0312	33	0.	.0	.0	.0	*	1	1312	133	0.	.1	.1	.1	*	1	2312	233	0.	.2	.2	.2
1	0318	34	0.	.0	.0	.0	*	1	1318	134	0.	.1	.1	.1	*	1	2318	234	0.	.2	.2	.2
1	0324	35	0.	.0	.0	.0	*	1	1324	135	0.	.1	.1	.1	*	1	2324	235	0.	.2	.2	.2
1	0330	36	0.	.0	.0	.0	*	1	1330	136	0.	.1	.1	.1	*	1	2330	236	0.	.2	.2	.2
1	0336	37	0.	.0	.0	.0	*	1	1336	137	0.	.1	.1	.1	*	1	2336	237	0.	.2	.2	.2
1	0342	38	0.	.0	.0	.0	*	1	1342	138	0.	.1	.1	.1	*	1	2342	238	0.	.2	.2	.2
1	0348	39	0.	.0	.0	.0	*	1	1348	139	0.	.1	.1	.1	*	1	2348	239	0.	.2	.2	.2
1	0354	40	0.	.0	.0	.0	*	1	1354	140	0.	.1	.1	.1	*	1	2354	240	0.	.2	.2	.2
1	0400	41	0.	.0	.0	.0	*	1	1400	141	0.	.1	.1	.1	*	2	0000	241	0.	.2	.2	.2
1	0406	42	0.	.0	.0	.0	*	1	1406	142	0.	.1	.1	.1	*	2	0006	242	0.	.2	.2	.2
1	0412	43	0.	.0	.0	.0	*	1	1412	143	0.	.1	.1	.1	*	2	0012	243	0.	.2	.2	.2
1	0418	44	0.	.0	.0	.0	*	1	1418	144	0.	.1	.1	.1	*	2	0018	244	0.	.2	.2	.2
1	0424	45	0.	.0	.0	.0	*	1	1424	145	0.	.1	.1	.1	*	2	0024	245	0.	.2	.2	.2
1	0430	46	0.	.0	.0	.0	*	1	1430	146	0.	.1	.1	.1	*	2	0030	246	0.	.2	.2	.2
1	0436	47	0.	.0	.0	.0	*	1	1436	147	0.	.1	.1	.1	*	2	0036	247	0.	.2	.2	.2
1	0442	48	0.	.0	.0	.0	*	1	1442	148	0.	.1	.1	.1	*	2	0042	248	0.	.2	.2	.2
1	0448	49	0.	.0	.0	.0	*	1	1448	149	0.	.1	.1	.1	*	2	0048	249	0.	.2	.2	.2
1	0454	50	0.	.0	.0	.0	*	1	1454	150	0.	.1	.1	.1	*	2	0054	250	0.	.2	.2	.2
1	0500	51	0.	.0	.0	.0	*	1	1500	151	0.	.1	.1	.1	*	2	0100	251	0.	.2	.2	.2
1	0506	52	0.	.0	.0	.0	*	1	1506	152	0.	.1	.1	.1	*	2	0106	252	0.	.2	.2	.2
1	0512	53	0.	.0	.0	.0	*	1	1512	153	0.	.1	.1	.1	*	2	0112	253	0.	.2	.2	.2
1	0518	54	0.	.0	.0	.0	*	1	1518	154	0.	.1	.1	.1	*	2	0118	254	0.	.2	.2	.2
1	0524	55	0.	.0	.0	.0	*	1	1524	155	0.	.1	.1	.1	*	2	0124	255	0.	.2	.2	.2
1	0530	56	0.	.0	.0	.0	*	1	1530	156	0.	.1	.1	.1	*	2	0130	256	0.	.2	.2	.2
1	0536	57	0.	.0	.0	.0	*	1	1536	157	0.	.1	.1	.1	*	2	0136	257	0.	.2	.2	.2
1	0542	58	0.	.0	.0	.0	*	1	1542	158	0.	.1	.1	.1	*	2	0142	258	0.	.2	.2	.2
1	0548	59	0.	.0	.0	.0	*	1	1548	159	0.	.1	.1	.1	*	2	0148	259	0.	.2	.2	.2
1	0554	60	0.	.0	.0	.0	*	1	1554	160	0.	.1	.1	.1	*	2	0154	260	0.	.2	.2	.2
1	0600	61	0.	.0	.0	.0	*	1	1600	161	0.	.1	.1	.1	*	2	0200	261	0.	.2	.2	.2
1	0606	62	0.	.0	.0	.0	*	1	1606	162	0.	.1	.1	.1	*	2	0206	262	0.	.2	.2	.2
1	0612	63	0.	.0	.0	.0	*	1	1612	163	0.	.1	.1	.1	*	2	0212	263	0.	.2	.2	.2
1	0618	64	0.	.0	.0	.0	*	1	1618	164	0.	.1	.1	.1	*	2	0218	264	0.	.2	.2	.2
1	0624	65	0.	.0	.0	.0	*	1	1624	165	0.	.1	.1	.1	*	2	0224	265	0.	.2	.2	.2
1	0630	66	0.	.0	.0	.0	*	1	1630	166	0.	.1	.1	.1	*	2	0230	266	0.	.2	.2	.2
1	0636	67	0.	.0	.0	.0	*	1	1636	167	0.	.1	.1	.1	*	2	0236	267	0.	.2	.2	.2
1	0642	68	0.	.0	.0	.0	*	1	1642	168	0.	.1	.1	.1	*	2	0242	268	0.	.2	.2	.2
1	0648	69	0.	.0	.0	.0	*	1	1648	169	0.	.1	.1	.1	*	2	0248	269	0.	.2	.2	.2
1	0654	70	0.	.0	.0	.0	*	1	1654	170	0.	.1	.1	.1	*	2	0254	270	0.	.2	.2	.2
1	0700	71	0.	.0	.0	.0	*	1	1700	171	0.	.1	.1	.1	*	2	0300	271	0.	.2	.2	.2
1	0706	72	0.	.0	.0	.0	*	1	1706	172	0.	.1	.1	.1	*	2	0306	272	0.	.2	.2	.2
1	0712	73	0.	.0	.0	.0	*	1	1712	173	0.	.1	.1	.1	*	2	0312	273	0.	.2	.2	.2
1	0718	74	0.	.0	.0	.0	*	1	1718	174	0.	.1	.1	.1	*	2	0318	274	0.	.2	.2	.2
1	0724	75	0.	.0	.0	.0	*	1	1724	175	0.	.1	.1	.1	*	2	0324	275	0.	.2	.2	.2
1	0730	76	0.	.0	.0	.0	*	1	1730	176	0.	.1	.1	.1	*	2	0330	276	0.	.2	.2	.2
1	0736	77	0.	.0	.0	.0	*	1	1736	177	0.	.1	.1	.1	*	2	0336	277	0.	.2	.2	.2
1	0742	78	0.	.0	.0	.0	*	1	1742	178	0.	.1	.1	.1	*	2	0342	278	0.	.2		

1	0842	88	0.	.0	.0 *	1	1842	188	0.	.1	.1 *	2	0442	288	0.	.2	.2
1	0848	89	0.	.0	.0 *	1	1848	189	0.	.1	.1 *	2	0448	289	0.	.2	.2
1	0854	90	0.	.0	.0 *	1	1854	190	0.	.1	.1 *	2	0454	290	0.	.2	.2
1	0900	91	0.	.0	.0 *	1	1900	191	0.	.1	.1 *	2	0500	291	0.	.2	.2
1	0906	92	0.	.0	.0 *	1	1906	192	0.	.1	.1 *	2	0506	292	0.	.2	.2
1	0912	93	0.	.0	.0 *	1	1912	193	0.	.1	.1 *	2	0512	293	0.	.2	.2
1	0918	94	0.	.0	.0 *	1	1918	194	0.	.1	.1 *	2	0518	294	0.	.2	.2
1	0924	95	0.	.0	.0 *	1	1924	195	0.	.1	.1 *	2	0524	295	0.	.2	.2
1	0930	96	0.	.0	.0 *	1	1930	196	0.	.1	.1 *	2	0530	296	0.	.2	.2
1	0936	97	0.	.0	.0 *	1	1936	197	0.	.1	.1 *	2	0536	297	0.	.2	.2
1	0942	98	0.	.0	.0 *	1	1942	198	0.	.1	.1 *	2	0542	298	0.	.2	.2
1	0948	99	0.	.0	.0 *	1	1948	199	0.	.1	.1 *	2	0548	299	0.	.2	.2
1	0954	100	0.	.0	.0 *	1	1954	200	0.	.1	.1 *	2	0554	300	0.	.2	.2

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	29.90-HR
+	(CFS)	(HR)					
+	0.	.00					
		(CFS)					
		(INCHES)	.000	.000	.000	.000	.000
		(AC-FT)	0.	0.	0.	0.	0.
PEAK STORAGE	TIME		6-HR	MAXIMUM AVERAGE STORAGE	24-HR	72-HR	29.90-HR
+	(AC-FT)	(HR)					
	0.	23.90	0.	0.	0.	0.	0.
PEAK STAGE	TIME		6-HR	MAXIMUM AVERAGE STAGE	24-HR	72-HR	29.90-HR
+	(FEET)	(HR)					
	.16	24.30	.16	.11	.09	.09	.09
CUMULATIVE AREA =			.00 SQ MI				

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	1.00
HYDROGRAPH AT					
+	SUB1	.03	1	FLOW TIME	16. 11.70
ROUTED TO					
+	RSUB1	.03	1	FLOW TIME	15. 11.70
HYDROGRAPH AT					
+	SUB2	.01	1	FLOW TIME	6. 11.70
2 COMBINED AT					
+	CWPS1	.04	1	FLOW TIME	20. 11.70
HYDROGRAPH AT					
+	SLLE	.03	1	FLOW TIME	14. 11.80
ROUTED TO					
+	RSLLLE	.03	1	FLOW TIME	13. 11.80
2 COMBINED AT					
+	CWPS	.07	1	FLOW TIME	32. 11.70
ROUTED TO					
+	RCWPS	.07	1	FLOW TIME	0. .00
** PEAK STAGES IN FEET **					
			1	STAGE TIME	2.59 24.40
HYDROGRAPH AT					
+	SUB2A	.02	1	FLOW TIME	9. 11.70
ROUTED TO					
+	RSUB2A	.02	1	FLOW TIME	9. 11.80
HYDROGRAPH AT					
+	SUB2B	.01	1	FLOW	5.

				TIME	11.70
HYDROGRAPH AT					
+	SUB2C	.01	1	FLOW	4.
				TIME	11.70
ROUTED TO					
+	RSUB2C	.01	1	FLOW	4.
				TIME	11.70
HYDROGRAPH AT					
+	SUB3	.01	1	FLOW	4.
				TIME	11.80
4 COMBINED AT					
+	SWB1	.04	1	FLOW	21.
				TIME	11.70
ROUTED TO					
+	RSWB1	.04	1	FLOW	0.
				TIME	.00
				** PEAK STAGES IN FEET **	
			1	STAGE	1.69
				TIME	24.50
HYDROGRAPH AT					
+	SUB4	.03	1	FLOW	15.
				TIME	11.80
ROUTED TO					
+	RSUB4	.03	1	FLOW	15.
				TIME	11.80
HYDROGRAPH AT					
+	SUB5	.00	1	FLOW	3.
				TIME	11.70
ROUTED TO					
+	RSUB5	.00	1	FLOW	3.
				TIME	11.70
HYDROGRAPH AT					
+	SUB5A	.00	1	FLOW	2.
				TIME	11.70
3 COMBINED AT					
+	SWB4	.04	1	FLOW	17.
				TIME	11.80
ROUTED TO					
+	RSWB4	.04	1	FLOW	0.
				TIME	.00
				** PEAK STAGES IN FEET **	
			1	STAGE	1.52
				TIME	24.60
HYDROGRAPH AT					
+	SUB6	.02	1	FLOW	10.
				TIME	11.80
HYDROGRAPH AT					
+	SUB7	.01	1	FLOW	5.
				TIME	11.70
ROUTED TO					
+	RSUB7	.01	1	FLOW	5.
				TIME	11.70
2 COMBINED AT					
+	SWB5	.03	1	FLOW	15.
				TIME	11.70
ROUTED TO					
+	RSWB5	.03	1	FLOW	0.
				TIME	.00
				** PEAK STAGES IN FEET **	
			1	STAGE	1.15
				TIME	24.40
HYDROGRAPH AT					
+	SUB8	.00	1	FLOW	3.
				TIME	11.70
ROUTED TO					
+	RSUB8	.00	1	FLOW	3.
				TIME	11.70
HYDROGRAPH AT					
+	SUB9	.01	1	FLOW	4.
				TIME	11.70

```

ROUTED TO
+          RSUB9      .01    1  FLOW      4.
                                TIME     11.70

HYDROGRAPH AT
+          SUB10      .01    1  FLOW      3.
                                TIME     11.70

  3 COMBINED AT
+          SWB8       .02    1  FLOW     10.
                                TIME     11.70

ROUTED TO
+          RSWB8      .02    1  FLOW      0.
                                TIME      .00

                                ** PEAK STAGES IN FEET **
                                1  STAGE      .62
                                TIME     24.00

HYDROGRAPH AT
+          SUB12      .00    1  FLOW      2.
                                TIME     11.70

ROUTED TO
+          RSUB12     .00    1  FLOW      0.
                                TIME      .00

                                ** PEAK STAGES IN FEET **
                                1  STAGE      .12
                                TIME     24.10

HYDROGRAPH AT
+          SUB12A     .01    1  FLOW      3.
                                TIME     11.70

ROUTED TO
+          RSUB12A    .01    1  FLOW      0.
                                TIME      .00

                                ** PEAK STAGES IN FEET **
                                1  STAGE      .20
                                TIME     24.00

HYDROGRAPH AT
+          SUB3A      .00    1  FLOW      2.
                                TIME     11.70

ROUTED TO
+          RSUB3A     .00    1  FLOW      0.
                                TIME      .00

                                ** PEAK STAGES IN FEET **
                                1  STAGE      .16
                                TIME     24.30

HYDROGRAPH AT
+          SUB11      .02    1  FLOW      7.
                                TIME     11.90

```

1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		
							PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)
FOR PLAN = 1 RATIO= .00									
RSUB1	MANE	2.37	15.48	703.67	.72	6.00	14.70	702.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1056E+01 EXCESS= .0000E+00 OUTFLOW= .1056E+01 BASIN STORAGE= .2975E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RSLL	MANE	.83	13.55	708.84	.72	6.00	13.47	708.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1095E+01 EXCESS= .0000E+00 OUTFLOW= .1095E+01 BASIN STORAGE= .5327E-04 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RSUB2A	MANE	1.16	9.20	704.51	.72	6.00	9.02	708.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .7208E+00 EXCESS= .0000E+00 OUTFLOW= .7208E+00 BASIN STORAGE= .1059E-03 PERCENT ERROR= .0									
FOR PLAN = 1 RATIO= .00									
RSUB2C	MANE	1.15	3.86	703.26	.72	6.00	3.82	702.00	.72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2467E+00 EXCESS= .0000E+00 OUTFLOW= .2467E+00 BASIN STORAGE= .8328E-04 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
RSUB4 MANE .37 14.82 708.41 .72 6.00 14.73 708.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1253E+01 EXCESS= .0000E+00 OUTFLOW= .1253E+01 BASIN STORAGE= .3586E-04 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
RSUB5 MANE .66 2.69 701.79 .72 6.00 2.69 702.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1657E+00 EXCESS= .0000E+00 OUTFLOW= .1657E+00 BASIN STORAGE= .4239E-04 PERCENT ERROR= .0

FOR PLAN = 1 RATIO= .00
RSUB7 MANE 2.44 5.12 703.56 .72 6.00 4.92 702.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3468E+00 EXCESS= .0000E+00 OUTFLOW= .3469E+00 BASIN STORAGE= .2294E-03 PERCENT ERROR= -.1

FOR PLAN = 1 RATIO= .00
RSUB8 MANE 1.48 2.66 702.00 .72 6.00 2.66 702.00 .73

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1696E+00 EXCESS= .0000E+00 OUTFLOW= .1696E+00 BASIN STORAGE= .1069E-03 PERCENT ERROR= -.1

FOR PLAN = 1 RATIO= .00
RSUB9 MANE .39 4.03 702.18 .72 6.00 4.03 702.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2505E+00 EXCESS= .0000E+00 OUTFLOW= .2505E+00 BASIN STORAGE= .2838E-04 PERCENT ERROR= .0

*** NORMAL END OF HEC-1 ***

Attachment 5
Updated Runoff and Run-On Hydraulic Calculations

Worksheets

Worksheet for CWPS Ditch Capacity

Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	5.00 %
Normal Depth	1.00 ft
Left Side Slope	3.00 H:V
Right Side Slope	15.00 H:V
Results	
Discharge	74.81 cfs
Flow Area	9.0 ft ²
Wetted Perimeter	18.2 ft
Hydraulic Radius	0.49 ft
Top Width	18.00 ft
Critical Depth	1.34 ft
Critical Slope	1.06 %
Velocity	8.31 ft/s
Velocity Head	1.07 ft
Specific Energy	2.07 ft
Froude Number	2.072
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.00 ft
Critical Depth	1.34 ft
Channel Slope	5.00 %
Critical Slope	1.06 %

Worksheet for V-DITCH 1 Capacity

Project Description	
Friction Method	Manning
Solve For	Formula Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	2.00 %
Normal Depth	1.00 ft
Left Side Slope	20.00 H:V
Right Side Slope	2.00 H:V
Results	
Discharge	57.79 cfs
Flow Area	11.0 ft ²
Wetted Perimeter	22.3 ft
Hydraulic Radius	0.49 ft
Top Width	22.00 ft
Critical Depth	1.11 ft
Critical Slope	1.12 %
Velocity	5.25 ft/s
Velocity Head	0.43 ft
Specific Energy	1.43 ft
Froude Number	1.310
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.00 ft
Critical Depth	1.11 ft
Channel Slope	2.00 %
Critical Slope	1.12 %

Worksheet for V-DITCH 2 (MIN SLOPE) Capacity

Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	0.30 %
Normal Depth	1.50 ft
Left Side Slope	20.00 H:V
Right Side Slope	20.00 H:V
Results	
Discharge	120.83 cfs
Flow Area	45.0 ft ²
Wetted Perimeter	60.1 ft
Hydraulic Radius	0.75 ft
Top Width	60.00 ft
Critical Depth	1.18 ft
Critical Slope	1.09 %
Velocity	2.69 ft/s
Velocity Head	0.11 ft
Specific Energy	1.61 ft
Froude Number	0.547
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	0.00 ft/s
Upstream Velocity	0.00 ft/s
Normal Depth	1.50 ft
Critical Depth	1.18 ft
Channel Slope	0.30 %
Critical Slope	1.09 %

Worksheet for V-DITCH 2 (MAX SLOPE) Capacity

Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	2.50 %
Normal Depth	1.00 ft
Left Side Slope	4.00 H:V
Right Side Slope	3.00 H:V
Results	
Discharge	20.18 cfs
Flow Area	3.5 ft ²
Wetted Perimeter	7.3 ft
Hydraulic Radius	0.48 ft
Top Width	7.00 ft
Critical Depth	1.16 ft
Critical Slope	1.15 %
Velocity	5.76 ft/s
Velocity Head	0.52 ft
Specific Energy	1.52 ft
Froude Number	1.437
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.00 ft
Critical Depth	1.16 ft
Channel Slope	2.50 %
Critical Slope	1.15 %

Worksheet for V-DITCH 3 Capacity

Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	2.00 %
Normal Depth	1.00 ft
Left Side Slope	25.00 H:V
Right Side Slope	2.50 H:V
Results	
Discharge	72.44 cfs
Flow Area	13.8 ft ²
Wetted Perimeter	27.7 ft
Hydraulic Radius	0.50 ft
Top Width	27.50 ft
Critical Depth	1.12 ft
Critical Slope	1.12 %
Velocity	5.27 ft/s
Velocity Head	0.43 ft
Specific Energy	1.43 ft
Froude Number	1.313
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.00 ft
Critical Depth	1.12 ft
Channel Slope	2.00 %
Critical Slope	1.12 %

Worksheet for BERM 2 Capacity

Project Description	
Friction Method	Manning
Solve For	Formula Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	1.60 %
Normal Depth	1.50 ft
Left Side Slope	2.50 H:V
Right Side Slope	2.50 H:V
Bottom Width	10.00 ft
Results	
Discharge	169.31 cfs
Flow Area	20.6 ft ²
Wetted Perimeter	18.1 ft
Hydraulic Radius	1.14 ft
Top Width	17.50 ft
Critical Depth	1.77 ft
Critical Slope	0.86 %
Velocity	8.21 ft/s
Velocity Head	1.05 ft
Specific Energy	2.55 ft
Froude Number	1.333
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.50 ft
Critical Depth	1.77 ft
Channel Slope	1.60 %
Critical Slope	0.86 %

Worksheet for BERM 4 Capacity

Project Description	
Friction Method	Manning Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	0.35 %
Normal Depth	1.00 ft
Left Side Slope	8.00 H:V
Right Side Slope	6.00 H:V
Results	
Discharge	15.40 cfs
Flow Area	7.0 ft ²
Wetted Perimeter	14.1 ft
Hydraulic Radius	0.49 ft
Top Width	14.00 ft
Critical Depth	0.79 ft
Critical Slope	1.26 %
Velocity	2.20 ft/s
Velocity Head	0.08 ft
Specific Energy	1.08 ft
Froude Number	0.549
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	0.00 ft/s
Upstream Velocity	0.00 ft/s
Normal Depth	1.00 ft
Critical Depth	0.79 ft
Channel Slope	0.35 %
Critical Slope	1.26 %

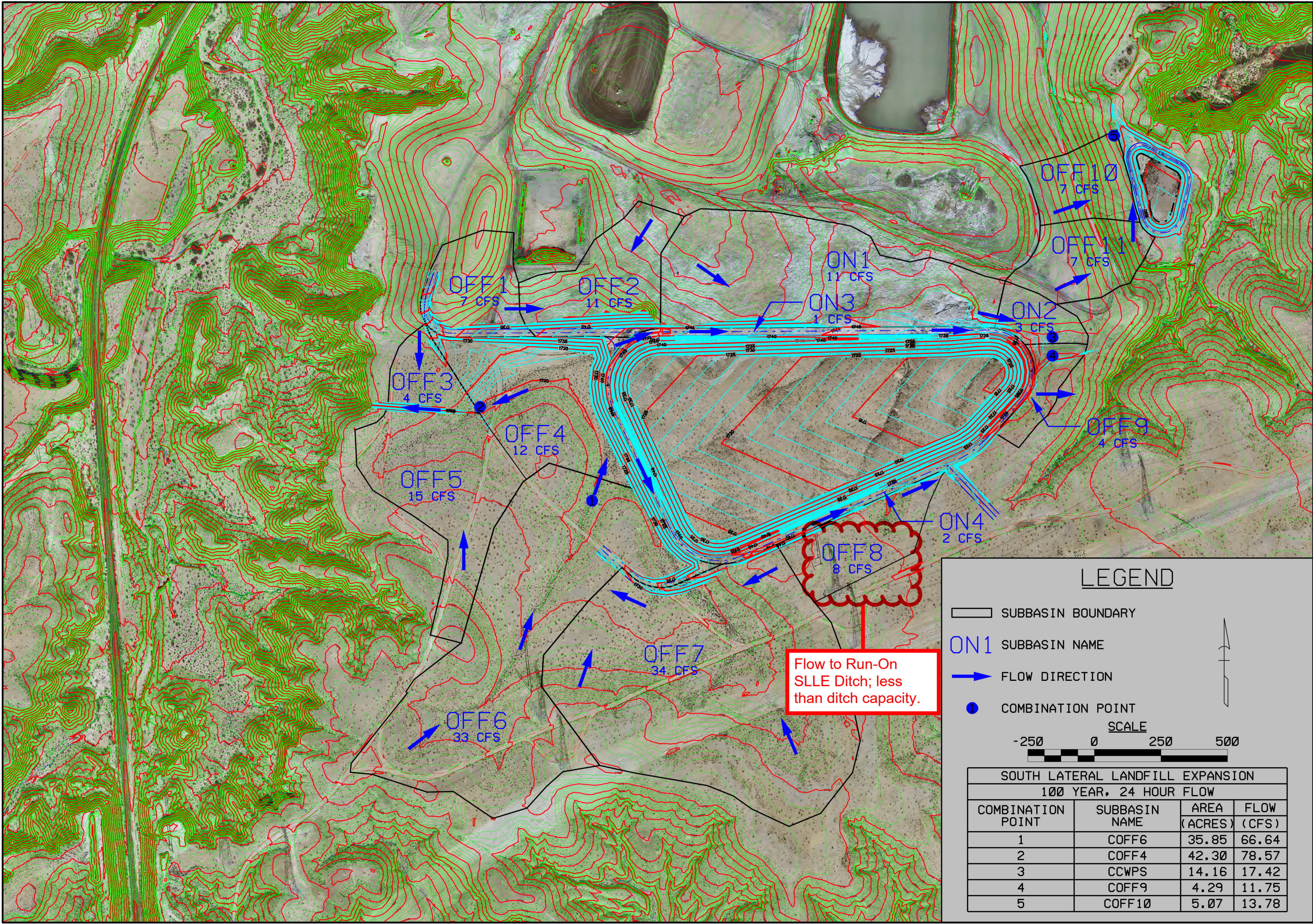
Worksheet for AZ CROSSING (SUB4) Capacity

Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	1.00 %
Normal Depth	1.00 ft
Left Side Slope	5.00 H:V
Right Side Slope	5.00 H:V
Bottom Width	15.00 ft
Results	
Discharge	101.90 cfs
Flow Area	20.0 ft ²
Wetted Perimeter	25.2 ft
Hydraulic Radius	0.79 ft
Top Width	25.00 ft
Critical Depth	1.00 ft
Critical Slope	0.99 %
Velocity	5.10 ft/s
Velocity Head	0.40 ft
Specific Energy	1.40 ft
Froude Number	1.004
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.00 ft
Critical Depth	1.00 ft
Channel Slope	1.00 %
Critical Slope	0.99 %

Worksheet for Run-On SLLE Ditch Capacity

Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.025
Channel Slope	0.70 %
Normal Depth	1.00 ft
Left Side Slope	6.00 H:V
Right Side Slope	4.00 H:V
Results	
Discharge	15.45 cfs
Flow Area	5.0 ft ²
Wetted Perimeter	10.2 ft
Hydraulic Radius	0.49 ft
Top Width	10.00 ft
Critical Depth	0.90 ft
Critical Slope	1.22 %
Velocity	3.09 ft/s
Velocity Head	0.15 ft
Specific Energy	1.15 ft
Froude Number	0.771
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.00 ft
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	0.00 ft/s
Upstream Velocity	0.00 ft/s
Normal Depth	1.00 ft
Critical Depth	0.90 ft
Channel Slope	0.70 %
Critical Slope	1.22 %

SLLE Drainage Map



LEGEND

- SUBBASIN BOUNDARY
- ON1 SUBBASIN NAME
- FLOW DIRECTION
- COMBINATION POINT

SCALE

-250 0 250 500

SOUTH LATERAL LANDFILL EXPANSION			
100 YEAR, 24 HOUR FLOW			
COMBINATION POINT	SUBBASIN NAME	AREA (ACRES)	FLOW (CFS)
1	COFF6	35.85	66.64
2	COFF4	42.30	78.57
3	CCWPS	14.16	17.42
4	COFF9	4.29	11.75
5	COFF10	5.07	13.78

PRELIMINARY
NOT FOR
CONSTRUCTION

DSGN SWD
BY APVD
JG

DESCRIPTION
REVISION
CHK JG

DR SB

MS

NO DATE
NO. DATE
DSGN MS

2485 VILLAGE VIEW DRIVE, SUITE 350
HENDERSON, NEVADA 89074
PHONE: 702-389-6175

REID GARDNER STATION
Facility Improvement
Program
Moapa, Nevada

CH2MHILL
SUBBASIN MAP
SOUTH LATERAL LANDFILL EXPANSION
NVE

DATE
PROJ 387230
DWG
SHEET

PRELIMINARY - NOT FOR CONSTRUCTION

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Culvert Calculations

HY-8 Culvert Analysis Report

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 19 cfs

Maximum Flow: 19 cfs

Table 1 - Summary of Culvert Flows at Crossing: SWB5

Headwater Elevation (ft)	Total Discharge (cfs)	SWB5 Culvert Discharge (cfs)	Roadway Discharge (cfs)	Iterations
1741.25	0.00	0.00	0.00	1
1741.88	1.90	1.90	0.00	1
1742.16	3.80	3.80	0.00	1
1742.41	5.70	5.70	0.00	1
1742.62	7.60	7.60	0.00	1
1742.81	9.50	9.50	0.00	1
1742.99	11.40	11.40	0.00	1
1743.17	13.30	13.30	0.00	1
1743.35	15.20	15.20	0.00	1
1743.55	17.10	17.10	0.00	1
1743.76	19.00	19.00	0.00	1
1746.00	32.45	32.45	0.00	Overtopping

Rating Curve Plot for Crossing: SWB5

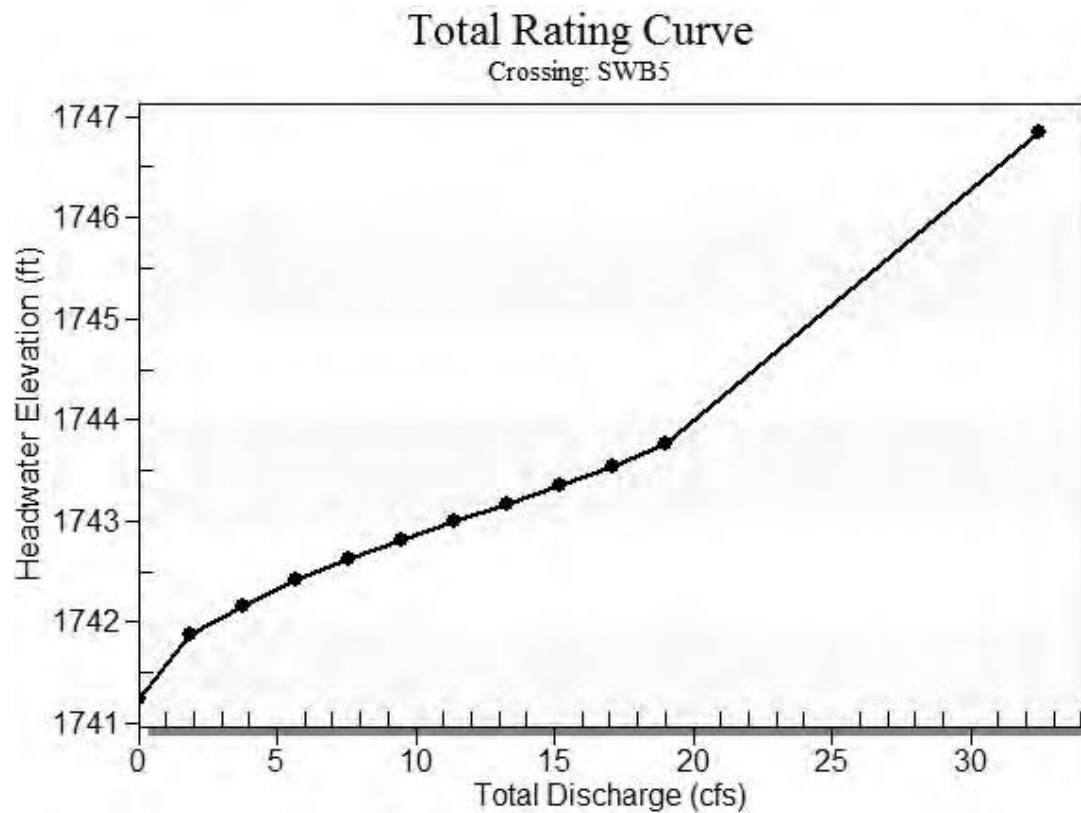


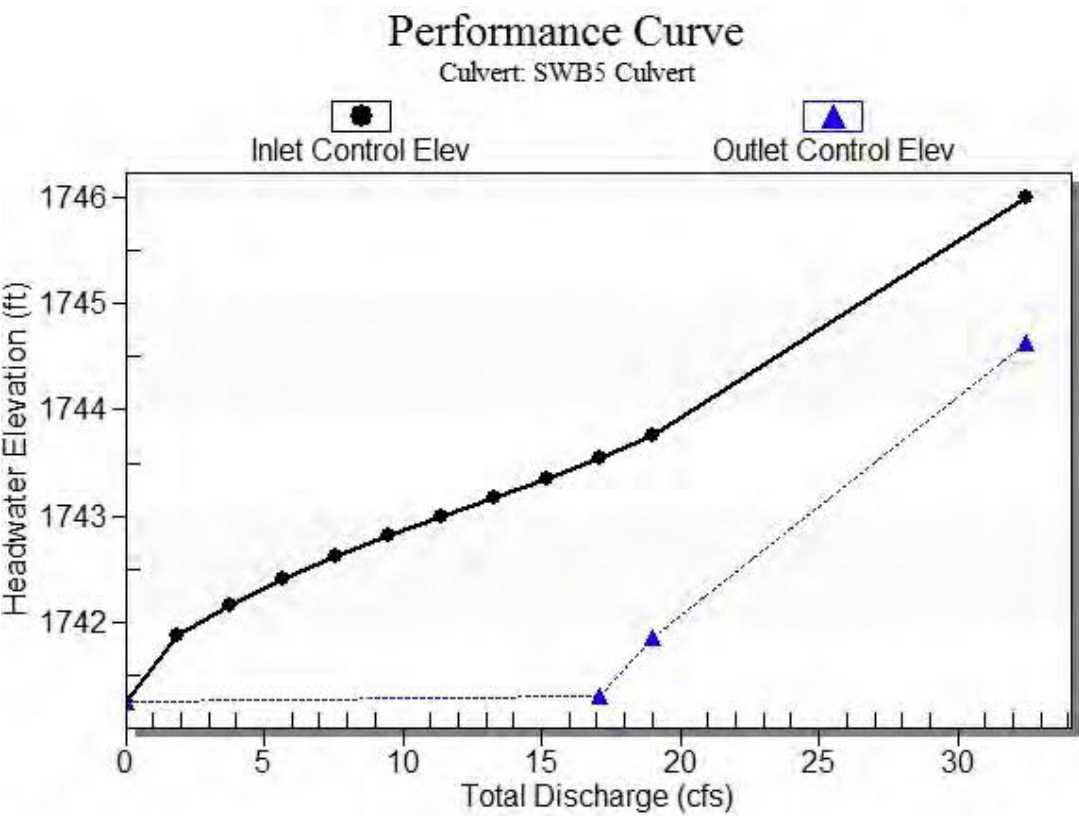
Table 2 - Culvert Summary Table: SWB5 Culvert

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.00	0.00	1741.25	0.000	0.000	0-NF	0.000	0.000	0.000	0.000	0.000	0.000
1.90	1.90	1741.88	0.632	0.0*	1-S2n	0.296	0.476	0.296	0.508	6.376	3.675
3.80	3.80	1742.16	0.914	0.0*	1-S2n	0.426	0.678	0.434	0.659	7.479	4.371
5.70	5.70	1742.41	1.159	0.0*	1-S2n	0.530	0.841	0.530	0.768	8.524	4.837
7.60	7.60	1742.62	1.374	0.0*	1-S2n	0.612	0.980	0.612	0.855	9.284	5.198
9.50	9.50	1742.81	1.563	0.0*	1-S2n	0.691	1.098	0.701	0.930	9.652	5.496
11.40	11.40	1742.99	1.740	0.0*	1-S2n	0.763	1.207	0.788	0.995	9.893	5.752
13.30	13.30	1743.17	1.916	0.0*	1-S2n	0.829	1.310	0.829	1.055	10.792	5.978
15.20	15.20	1743.35	2.099	0.0*	5-S2n	0.896	1.401	0.924	1.109	10.711	6.181
17.10	17.10	1743.55	2.297	0.058	5-S2n	0.958	1.489	0.988	1.159	11.052	6.366
19.00	19.00	1743.76	2.513	0.610	5-S2n	1.019	1.566	1.058	1.206	11.271	6.536

* Full Flow Headwater elevation is below inlet invert.

Straight Culvert
Inlet Elevation (invert): 1741.25 ft, Outlet Elevation (invert): 1738.72 ft
Culvert Length: 96.26 ft, Culvert Slope: 0.0263

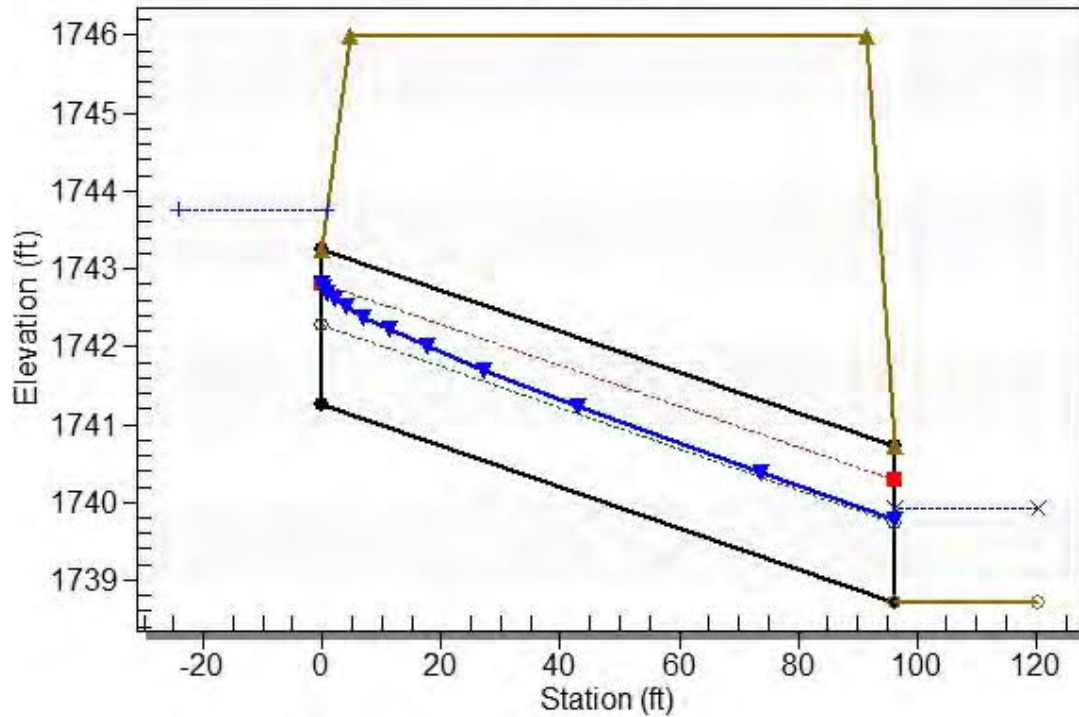
Culvert Performance Curve Plot: SWB5 Culvert



Water Surface Profile Plot for Culvert: SWB5 Culvert

Crossing - SWB5, Design Discharge - 19.0 cfs

Culvert - SWB5 Culvert, Culvert Discharge - 19.0 cfs



Site Data - SWB5 Culvert

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 1741.25 ft

Outlet Station: 96.23 ft

Outlet Elevation: 1738.72 ft

Number of Barrels: 1

Culvert Data Summary - SWB5 Culvert

Barrel Shape: Circular

Barrel Diameter: 2.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0130

Culvert Type: Straight

Inlet Configuration: Grooved End Projecting

Inlet Depression: NONE

Table 3 - Downstream Channel Rating Curve (Crossing: SWB5)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
0.00	1738.72	0.00	0.00	0.00	0.00
1.90	1739.23	0.51	3.68	1.52	1.28
3.80	1739.38	0.66	4.37	1.97	1.34
5.70	1739.49	0.77	4.84	2.30	1.38
7.60	1739.58	0.86	5.20	2.56	1.40
9.50	1739.65	0.93	5.50	2.78	1.42
11.40	1739.72	1.00	5.75	2.98	1.44
13.30	1739.77	1.05	5.98	3.16	1.45
15.20	1739.83	1.11	6.18	3.32	1.46
17.10	1739.88	1.16	6.37	3.47	1.47
19.00	1739.93	1.21	6.54	3.61	1.48

Tailwater Channel Data - SWB5

Tailwater Channel Option: Triangular Channel

Side Slope (H:V): 2.00 (1:1)

Channel Slope: 0.0480

Channel Manning's n: 0.0330

Channel Invert Elevation: 1738.72 ft

Roadway Data for Crossing: SWB5

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 20.00 ft

Crest Elevation: 1746.00 ft

Roadway Surface: Paved

Roadway Top Width: 87.00 ft

HY-8 Culvert Analysis Report

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 1 cfs

Design Flow: 10 cfs

Maximum Flow: 15 cfs

Table 1 - Summary of Culvert Flows at Crossing: SWB8 Culvert

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
1731.97	1.00	1.00	0.00	1
1732.27	2.40	2.40	0.00	1
1732.52	3.80	3.80	0.00	1
1732.73	5.20	5.20	0.00	1
1732.93	6.60	6.60	0.00	1
1733.14	8.00	8.00	0.00	1
1733.39	9.40	9.40	0.00	1
1733.50	10.00	10.00	0.00	1
1733.99	12.20	12.20	0.00	1
1734.37	13.60	13.60	0.00	1
1734.69	15.00	14.71	0.26	18
1734.66	14.61	14.61	0.00	Overtopping

Rating Curve Plot for Crossing: SWB8 Culvert

Total Rating Curve

Crossing: SWB8 Culvert

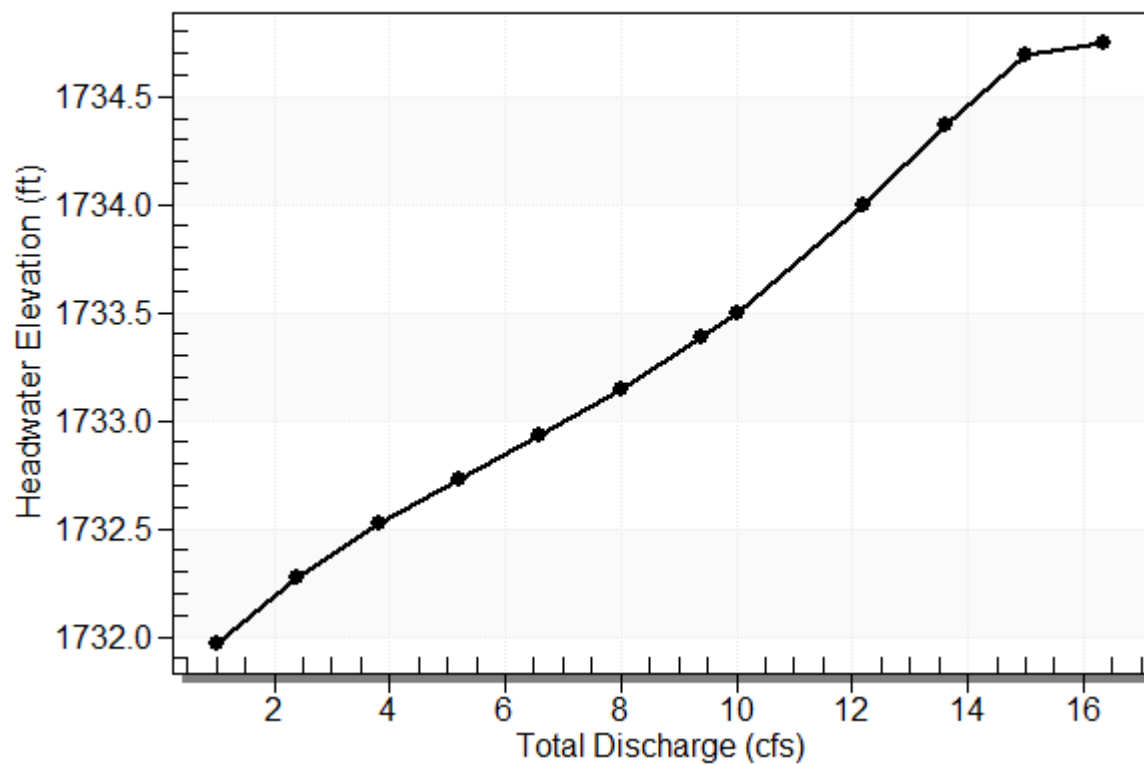


Table 2 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
1.00	1.00	1731.97	0.493	0.0*	1-S2n	0.224	0.369	0.232	0.037	5.653	3.344
2.40	2.40	1732.27	0.789	0.0*	1-S2n	0.345	0.582	0.356	0.062	7.280	4.699
3.80	3.80	1732.52	1.040	0.0*	1-S2n	0.436	0.745	0.453	0.082	8.175	5.618
5.20	5.20	1732.73	1.249	0.0*	1-S2n	0.513	0.876	0.535	0.099	8.880	6.340
6.60	6.60	1732.93	1.448	0.0*	1-S2n	0.584	0.991	0.605	0.114	9.568	6.942
8.00	8.00	1733.14	1.662	0.0*	5-S2n	0.650	1.092	0.650	0.128	10.549	7.464
9.40	9.40	1733.39	1.905	0.0*	5-S2n	0.712	1.183	0.746	0.141	10.364	7.933
10.00	10.00	1733.50	2.021	0.0*	5-S2n	0.739	1.218	0.768	0.146	10.625	8.122
12.20	12.20	1733.99	2.514	0.407	5-S2n	0.834	1.322	0.834	0.164	11.705	8.747
13.60	13.60	1734.37	2.885	0.935	5-S2n	0.895	1.370	0.948	0.175	11.206	9.108
15.00	14.71	1734.69	3.210	1.387	5-S2n	0.944	1.396	0.944	0.186	12.185	9.445

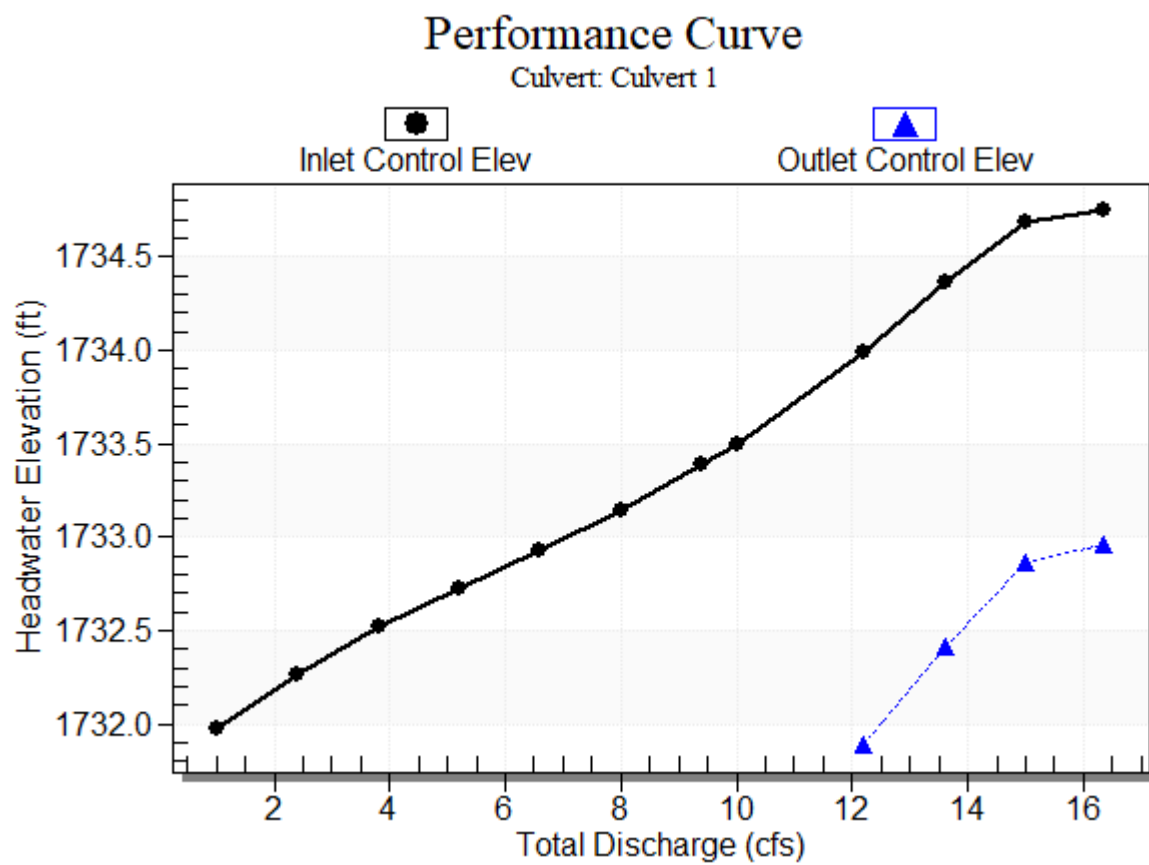
* Full Flow Headwater elevation is below inlet invert.

Straight Culvert

Inlet Elevation (invert): 1731.48 ft, Outlet Elevation (invert): 1728.40 ft

Culvert Length: 88.58 ft, Culvert Slope: 0.0348

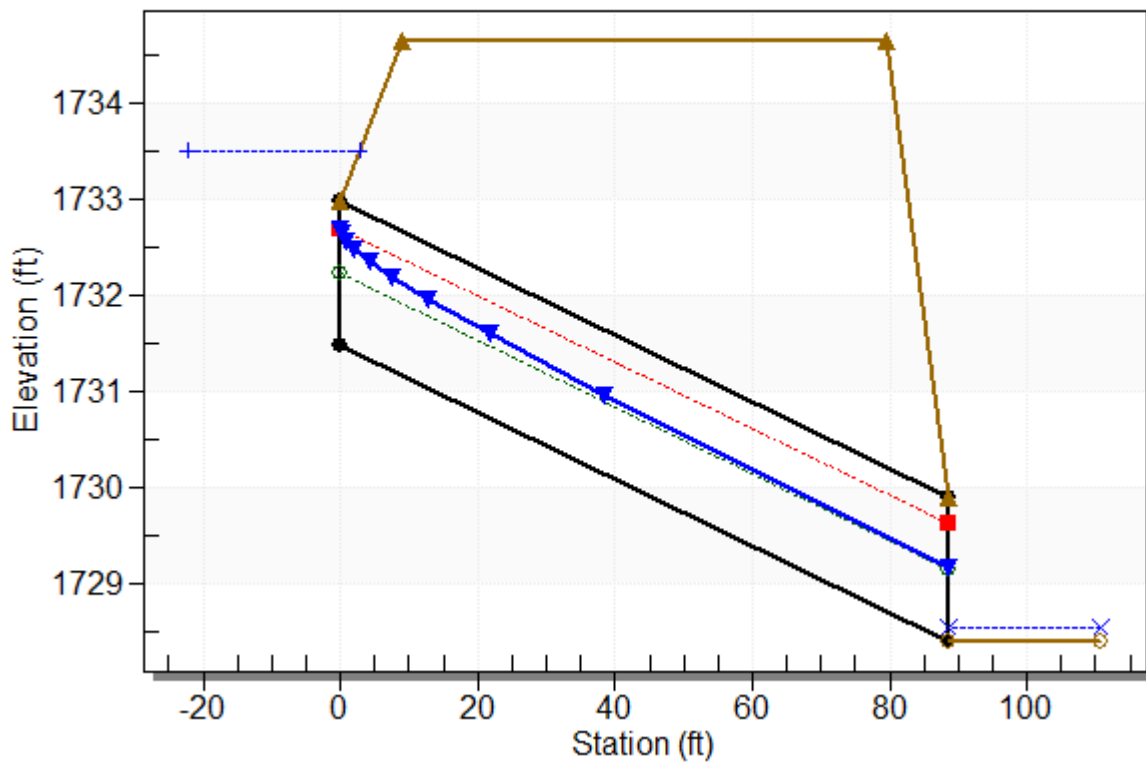
Culvert Performance Curve Plot: Culvert 1



Water Surface Profile Plot for Culvert: Culvert 1

Crossing - SWB8 Culvert, Design Discharge - 10.0 cfs

Culvert - Culvert 1, Culvert Discharge - 10.0 cfs



Site Data - Culvert 1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 1731.48 ft

Outlet Station: 88.53 ft

Outlet Elevation: 1728.40 ft

Number of Barrels: 1

Culvert Data Summary - Culvert 1

Barrel Shape: Circular

Barrel Diameter: 1.50 ft

Barrel Material:

Embedment: 0.00 in

Barrel Manning's n: 0.0130

Culvert Type: Straight

Inlet Configuration: Grooved End Projecting

Inlet Depression: None

Table 3 - Downstream Channel Rating Curve (Crossing: SWB8 Culvert)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
1.00	1728.44	0.04	3.34	1.05	3.09
2.40	1728.46	0.06	4.70	1.78	3.35
3.80	1728.48	0.08	5.62	2.33	3.51
5.20	1728.50	0.10	6.34	2.81	3.62
6.60	1728.51	0.11	6.94	3.24	3.70
8.00	1728.53	0.13	7.46	3.64	3.76
9.40	1728.54	0.14	7.93	4.00	3.82
10.00	1728.55	0.15	8.12	4.15	3.84
12.20	1728.56	0.16	8.75	4.67	3.91
13.60	1728.58	0.18	9.11	4.98	3.95
15.00	1728.59	0.19	9.45	5.28	3.99

Tailwater Channel Data - SWB8 Culvert

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 8.00 ft

Side Slope (H:V): 3.00 (1:1)

Channel Slope: 0.4560

Channel Manning's n: 0.0330

Channel Invert Elevation: 1728.40 ft

Roadway Data for Crossing: SWB8 Culvert

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 20.00 ft

Crest Elevation: 1734.66 ft

Roadway Surface: Gravel

Roadway Top Width: 70.41 ft

SWB4 Exhibit



UPDATED SWB4 STORMWATER BASIN FOOTPRINT

REID GARDNER STATION

NOTES:

BERM ELEVATION: 1731.50'
FILL VOLUME: 4,858 C. Y.

DATE: 10/14/2021